# Appendix H <br> Traffic, Transport and Accessibility 



## South Dural Urban Release Area

Traffic, Transport and Accessibility Assessment
South Dural Residents and Ratepayers Group
15 January 2009

# South Dural Urban Release Area 

Prepared for

## South Dural Residents and Ratepayers Group

Prepared by

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### 1.0 Introduction

### 1.1 Background

Maunsell Australia Pty Ltd (Maunsell) has been appointed by South Dural Residents and Ratepayers Group Incorporated (SDRRG) to prepare a Traffic Impact Assessment (TIA) for the proposed rezoning of 240 ha of land at South Dural. The TIA will assist in the development of a Concept Structure Plan for the Department of Planning (DoP).

The South Dural Urban Release Area (South Dural) is located within the Local Government Area (LGA) of Hornsby. Old Northern Road to the west of the study area forms the boundary of Hornsby LGA and Baulkham Hills LGA. The release area is approximately 6 km to the north east of Baulkham Hills, 11 km to the west of Hornsby, and 25 km to the north west of Sydney. The study area is approximately 240 ha and the proposed housing strategy for the area would yield approximately 2,300 residential dwellings and a local neighbourhood centre. The study area in its regional context is shown in Figure 1.1.

Figure 1.1: Regional Context of South Dural Urban Release Area


Source: UBD, 2008

### 1.2 Purpose and Scope

The purpose of this report is to provide a review of the potential traffic impacts of the proposed 2,300 residential dwellings on the external road network. The assessment involves determining the level of trip generation associated with the proposed development within the study area and will provide recommendations on the traffic requirement as a result of the development trips. The report also provides a proposed access strategy for the study area and recommends the geometric layouts required for all access points.

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### 1.3 Report Framework

The report has been structured into the following sections:

- Section 2 details the existing transport conditions in the locality for all modes of transport;
- Section 3 reviews the impacts of changes to the traffic flow and road network, prior to future development of South Dural;
- Section 4 describes the details of the proposed development within the study area;
- Section 5 details the access strategy for the development;
- Section 6 provides a traffic impact assessment of the proposed development and surrounding road network and identifies appropriate transport infrastructure to cater for the forecast traffic flows; and
- Section 6 provides summary and conclusions of the report.


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### 2.0 Existing conditions

### 2.1 Site Description

South Dural is located between Round Corner and Castle Hill, to the north west of Sydney. The site area is approximately 240 ha. The study area is surrounded by existing residential area in Castle Hill, Glenhaven and Cherrybrook. There is a small retail centre in Round Corner and a light industrial area to the east of the study area, north of Sebastian Drive.

The study area is bounded by Old Northern Road to the north, south and west and by Hastings Road and New Line Road to the east. The location of the site is shown in Figure 2.1.

Figure 2.1: Location of South Dural


Source: UBD, 2008
The site is currently occupied by rural residential dwellings with the remaining area being open grass land and woodland. Most of the current dwellings or properties have direct access to Old Northern Road or New Line Road. Wayfield Road and Franlee Road form the current internal road network of the site as well as providing access to some properties from Old Northern Road.

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### 2.2 Travel Behaviour

### 2.2.1 Journey to Work data

Journey to Work data (JTW) includes details of the origin and destination of trips, together with characteristics of the journey such as mode of travel. 2006 JTW data has been analysed to determine the current distribution of trips from the study area.

South Dural is located on the boundary of the Baulkham Hills and Hornsby LGAs. Therefore, the existing (2006) JTW data for all modes of transport from Baulkham Hills and Hornsby LGA has been analysed and is summarised in Table 2.1 and Table 2.2.

Table 2.1: Journey to Work destinations from Baulkham Hills LGA (all modes)

| Destination | Total Trips | Proportion |
| :---: | :---: | :---: |
| Baulkham Hills | 23,890 | $31 \%$ |
| Parramatta | 9,204 | $12 \%$ |
| Sydney | 8,309 | $11 \%$ |
| Blacktown | 6,218 | $8 \%$ |
| Hornsby | 4,440 | $6 \%$ |
| Ryde | 4,174 | $5 \%$ |
| Auburn | 2,250 | $3 \%$ |
| Holroyd | 1,800 | $2 \%$ |
| Willoughby | 1,760 | $2 \%$ |
| North Sydney | 1,760 | $2 \%$ |
| Fairfield | 1,397 | $16 \%$ |
| Other Areas | 10,977 | $100 \%$ |
| Total | 76,179 | 2 |

Source: Journey to Work (2006)

Table 2.2: Journey to Work destinations by any mode from Hornsby LGA (all modes)

| Destination | Total Trips | Proportion |
| :---: | :---: | :---: |
| Hornsby | 20,901 | $30 \%$ |
| Sydney | 11,483 | $17 \%$ |
| Ryde | 5,461 | $8 \%$ |
| Baulkham Hills | 4,462 | $6 \%$ |
| Ku-ring-gai | 4,297 | $6 \%$ |
| Willoughby | 3,919 | $6 \%$ |
| Parramatta | 3,683 | $5 \%$ |
| North Sydney | 3,154 | $5 \%$ |
| Blacktown | 1,418 | $2 \%$ |
| Auburn | 1,264 | $2 \%$ |
| Warringah | 1,239 | $2 \%$ |
| Other Areas | 8,110 | $12 \%$ |
| Total | 69,391 | $100 \%$ |

Source: Journey to Work (2006)

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The JTW data for both LGAs shows that a high proportion of trips are self contained within the Baulkham Hills (31\%) and Hornsby (30\%). Other major destinations that people travelled to work include Parramatta, Sydney, Blacktown and Ryde.

### 2.3 Pedestrian and Cycle Facilities

There is a dedicated shared path on the western side of Old Northern Road between Glenhaven Road and Kenthurst Road, as shown in Figure 2.2. Cyclists are allowed to cycle on road along other sections of Old Northern Road and New Line Road in the vicinity of the study area.

Pedestrian footpaths are provided on the northern end of Old Northern Road between Kenthurst Road and New Line Road, in the vicinity of Round Corner. There are also footpaths along New Line Road near the commercial areas and function centre to the east of the study area. No footpaths are provided on Hastings Road.

There are currently no dedicated pedestrian crossings at Old Northern Road, New Line Road and Hastings Road, except at the intersection of Old Northern Road/Kenthurst Road.

Figure 2.2: Cycle Map


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### 2.4 Bus Facilities

Hillsbus runs 7 public bus routes (Routes 636 -641) that currently service the study area and its vicinity. The bus routes and bus stop locations in the vicinity of the site are shown in Figure 2.3.

Figure 2.3: Bus routes and bus stop locations


Source: Maunsell 2008
The bus routes travel along Old Northern Road, New Line Road and Hastings Road and there are bus stops located along the length of these roads. The details and service frequencies of each bus route are summarised in Table 2.3.

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Table 2.3: Frequency of bus routes

| Route and bus service | AM peak (0700-0900) |  |  | PM peak (1600-1800) |  |  | Off peak (1000-1500) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Weekday | Sat | Sun | Weekday | Sat | Sun | Weekday | Sat | Sun |
| 636 Glenhaven to Castle Hill and Pennant Hills | 4 | 1 | 1 | 3 | 2 | 2 | 5 | 5 | 5 |
| 636 Pennant Hills and Castle Hill to Glenhaven | 0 | 1 | 0 | 4 | 2 | 3 | 6 | 5 | 5 |
| 637 Dural and Round Corner to Castle Hill and Pennant Hills | 1 | - | - | 0 | - | - | 0 | - | - |
| 637 Glenorie to Castle Hill and Pennant Hills (via Old Northern Rd) | 5 | 1 | 1 | 2 | 0 | 1 | 3 | 3 | 2 |
| 637 Glenorie to Castle Hill and Pennant Hills (via New Line Rd) | 2 | - | - | 0 | - | - | 0 | - | - |
| 637 Pennant Hills and Castle Hill to Glenorie (via Old Northern Rd) | 2 | 0 | 0 | 3 | 1 | 1 | 3 | 2 | 2 |
| 637 Pennant Hills and Castle Hill to Glenorie (via New Line Rd) | 0 | - | - | 3 | - | - | 1 | - | - |
| 638 Berrilee and Galston to Castle Hill and Pennant Hills (via Old Northern Rd) | 3 | 1 | 0 | 1 | 0 | 1 | 0 | 2 | 0 |
| 638 Berrilee and Galston to Castle Hill and Pennant Hills (via New Line Rd) | 2 | - | - | 1 | - | - | - | 0 | - |
| 638 Pennant Hills and Castle Hill to Galston and Berrilee (via Old Northern Rd) | 2 | 0 | 0 | 6 | 1 | 1 | 3 | 4 | 2 |
| 638 Pennant Hills and Castle Hill to Galston and Berrilee (via New Line Rd) | 1 | - | - | 1 | - | - | 0 | - | - |
| 639 Pitt Town Rd, Kenthurst to <br> Castle Hill and Pennant Hills (via Old <br> Northern Rd) | 3 | 1 | - | 0 | 0 | - | 1 | 2 | - |
| 639 Pennant Hills and Castle Hill to Pitt Town Rd, Kenthurst | 0 | 0 | - | 4 | 0 | - | 1 | 2 | - |
| 640 Porters Rd, Kenthurst to Castle <br> Hill and Pennant Hills (via Old <br> Northern Rd) | 3 | 1 | - | 1 | 0 | - | 1 | 2 | - |
| 640 Porters Rd, Kenthurst to Castle Hill and Pennant Hills (via New Line Rd) | 1 | - | - | 0 | - | - | 0 | - | - |
| 640 Pennant Hills and Castle Hill to Porters Rd, Kenthurst (via Old Northern Rd) | 0 | 0 | - | 4 | 0 | - | 1 | 2 | - |
| 641 Annangrove Rd to Castle Hill and Pennant Hills | 2 | 1 | - | 1 | 0 | - | 2 | 1 | - |
| 641 Pennant Hills and Castle Hill to Annangrove Rd | 0 | 0 | - | 4 | 0 | - | 1 | 2 | - |

Source: Hillsbus.com.au, 2008
The bus timetable shows that the study area is well serviced in the weekday peak hours however there are limited bus services on the weekend.

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### 2.5 Rail Facilities

The closest train station is located approximately 10km east of the study area at Pennant Hills. Pennant Hills station is serviced by the Northern Line which runs between Hornsby and North Sydney via Strathfield. The bus routes described in Section 2.4 connect Pennant Hills station to the study area and are timetabled to coincide with the train services. The frequency of trains servicing Pennant Hills station is shown in Table 2.4.

Table 2.4: Frequency of train services at Pennant Hills Station

| Direction | AM peak (0700-0900) |  |  | PM peak (1600-1800) |  |  | Off peak (1000-1500) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Weekday | Sat | Sun | Weekday | Sat | Sun | Weekday | Sat | Sun |
| Pennant Hill to City | 7 | 4 | 4 | 7 | 4 | 4 | 11 | 10 | 10 |
| City to Pennant Hills | 7 | 4 | 4 | 10 | 4 | 4 | 11 | 10 | 10 |

Source: Cityrail.info, 2008
During the weekdays, rail services operate between Pennant Hills and City at approximately every 15 minutes in the AM peak Additional outbound train services operate during the PM peak from City to Pennant Hills. During the weekday off peak hours and weekends, trains run in either direction at approximately every 30 minutes.

### 2.6 Road Network

The main roads in the vicinity of the study area are Old Northern Road, New Line Road and Hastings Road. Glenhaven Road and Gilbert Road connect to Old Northern Road and provide links to the surrounding areas including Castle Hill and Kellyville.

### 2.6.1 Old Northern Road

Old Northern Road runs along the southern, western and northern boundary of the study area. It runs in north-south direction from Castle Hill Road to Kenthurst Road and in east-west direction between Kenthurst Road and New Line Road. Old Northern Road is a sealed road with predominantly one lane in each direction in the vicinity of the study area. However, the road widens to 2 lanes in each direction at the intersections. Gilbert Road and Kenthurst Road intersect with Old Northern Road and both intersections are controlled by traffic signals. New Line Road meets Old Northern Road at a two lane roundabout. Figure 2.4 shows a typical cross-section of Old Northern Road in the vicinity of the study area.

Figure 2.4: Old Northern Road south of Glenhaven Road (looking south)


Source: Maunsell, 2008

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The majority of the road in the vicinity of the study area has a speed limit of $60 \mathrm{~km} / \mathrm{hr}$, except there is a $40 \mathrm{~km} / \mathrm{hr}$ school zone south of Malabar Road. The school zone is operational on school days between 8 am and 9.30 am and 2.30 pm and 4 pm . Parking is not permitted along the section of the road bordering the site.

### 2.6.2 New Line Road

New Line Road is an arterial road that runs in a north-south direction from Old Northern Road at the north-eastern corner of the study area to the Cumberland Highway. It has one lane in each direction between Old Northern Road and Hastings Road. Roundabouts are located at the intersections of Old Northern Road, Sebastian Drive as well as the access to Bunnings. The speed limit along New Line Road is $60 \mathrm{~km} / \mathrm{hr}$. There are no parking lanes along the road. Figure 2.5 shows a section of New Line Road.

Figure 2.5: New Line Road approaching Old Northern Road roundabout (looking north)


Source: Maunsell, 2008

### 2.6.3 Hastings Road

Hastings Road is a 4 lane road that connects Old Northern Road to New Line Road. Parking is not permitted along its entire length and it has a speed limit of $60 \mathrm{~km} / \mathrm{hr}$. The intersection of Hastings Road and New Line Road is controlled by traffic signals. Figure 2.6 shows a section of Hastings Road.

Figure 2.6: Hastings Road north of Old Northern Road (looking north)


Source: Maunsell, 2008

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### 2.6.4 Glenhaven Road

Glenhaven road provides access to Glenhaven and Kellyville to the west of the study area. At the section east of Evans Road, it has one lane in each direction with a parking lane on the southern side of the carriageway. It has a speed limit of $60 \mathrm{~km} / \mathrm{hr}$ and a load limit of 8 tonnes. Glenhaven Road connects with Old Northern Road at a priority-controlled intersection.

### 2.6.5 Gilbert Road

Gilbert Road is a collector road that provides a link between Old Northern Road and Showground Road to the south west of the study area. It has a 4 lane carriageway with two trafficable lanes and two parking lanes. It has a speed limit of $60 \mathrm{~km} / \mathrm{hr}$. Vehicles 8 tonnes and over are not permitted to use Gilbert Road.

### 2.7 Traffic Volumes

### 2.7.1 Daily Traffic Counts

RTA Traffic Volume Data has been obtained to determine the historical traffic growth and current midblock traffic flows in the surrounding area. Table 2.5 shows historical Average Annual Daily Traffic (AADT) volumes at stations in the vicinity of the proposed development site. The location of the stations and the AADT for 2005 at these stations is shown in Figure 2.7.

Table 2.5: Historical Traffic Volumes and Growth

| Station Number | Location | 1993 | 1996 | 1999 | $\mathbf{2 0 0 2}$ | $\mathbf{2 0 0 5}$ | \% growth |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 73118 | Old Northern Road | 14975 | 15787 | 17332 | 18630 | 19269 | $\mathbf{2 . 1 2 \%}$ |
| 74446 | New Line Road | 21649 | 24131 | 26378 | 26261 | 26382 | $\mathbf{1 . 6 6 \%}$ |
| 73075 | Old Northern Road | 18844 | 20783 | 22844 | 23210 | 24739 | $\mathbf{2 . 2 9 \%}$ |
| 72047 | Old Northern Road | - | 45557 | 43025 | 47609 | 48217 | $\mathbf{0 . 6 3 \%}$ |

Source: RTA Traffic Volume Data

The data shows that between 1993 and 2005, there has been an average yearly growth rate of $1.68 \%$ in the surrounding area.

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Figure 2.7: 2005 AADT data in the vicinity of the site


Source: RTA Traffic Volume Data 2005
The 2005 AADT shows that there were approximately 19,000 vehicles per day on Old Northern Road and approximately 26,000 vehicles daily on New Line Road in the vicinity of the study area.

### 2.7.2 Intersection counts

Manual traffic counts were undertaken by Australasian Traffic Surveys (ATS) at the AM (6:30 to 9:30) and PM (16:30 to 18:30) peak hours on $15^{\text {th }}$ May 2008 at the following intersections:

- Old Northern Road / Glenhaven Road;
- Old Northern Road / Gilbert Road;
- Old Northern Road / Hastings Road; and
- New Line Road / Hastings Road.

Traffic counts at the intersection of Old Northern Road and New Line Road were taken on $6{ }^{\text {th }}$ June 2008 during the same peak hour time period. SCATS data for the signalised intersection of Old Northern Road and Kenthurst Road (TCS site 2954) on $15^{\text {th }}$ May 2008 was provided by the RTA.

Analysis of the data shows that the AM peak period for the network was between 7.30am and 8.30am and the PM peak was between 5 pm and 6 pm .

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The surveyed traffic data and a summary of the network turning traffic flows within the study area are included as Appendix A.

The latest traffic data shows that traffic flow in the peak hour traffic direction on Old Northern Road is approaching $1,000 \mathrm{veh} / \mathrm{hr}$ and that of New Line Road has exceeded 1,000 veh/hr. This implies that both roads are approaching capacity for a two lane road during the peak hours. Traffic flow on Hastings Road is relatively low (approximately 500 to $700 \mathrm{veh} / \mathrm{hr}$ ) compared to the rest of the road network surrounding the study area.

### 2.8 Intersection Assessment

Intersection assessment based on the surveyed traffic data has been carried out using SIDRA 3.2, a computer based modelling package which calculates isolated intersection performance.

The main performance indicators for SIDRA 3.2 include:

- Degree of saturation (DoS) - a measure of the ratio between traffic volumes and the capacity of the intersection;
- Average delay - how long in seconds the average vehicle waits at the intersection; and
- Level of service (LoS) - a measure of the overall performance of the intersection (Table 2.6).

Table 2.6: Performance Criteria for Intersections

| Level of <br> Service | Average <br> Delay <br> (secs/veh) | Traffic Signals and Roundabouts | Give Way and Stop Signs |
| :---: | :---: | :---: | :---: |
| A | Less than 14 | Good Operation | Good Operation |
| B | 15 to 28 | Good with acceptable delays and spare capacity | Acceptable delays and spare capacity |
| C | 29 to 42 | Satisfactory | Satisfactory, but accident study required |
| D | 43 to 56 | Operating near capacity | Near capacity and accident study required |
| E | 57 to 70 | At capacity; at signals incidents will cause <br> excessive delays | At capacity; requires other control mode |
| F | $>70$ | Roundabouts require other control mode | At capacity; requires other control mode |

Source: Guide to Traffic Generating Developments, RTA, 2002
The existing performance of the following intersections has been assessed:

- Old Northern Road / Hastings Road (priority controlled T intersection);
- Old Northern Road / Gilbert Road (signalised intersection);
- Old Northern Road / Glenhaven Road (priority controlled T intersection);
- Old Northern Road / Kenthurst Road (signalised intersection);
- Old Northern Road / New Line Road (roundabout); and
- New Line Road / Hastings Road (signalised intersection).

Table 2.7 summarises the existing intersection operation in the AM peak period and Table 2.8 summarises the existing intersection operation of the PM peak period. More detailed results are presented as Appendix B.

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Table 2.7: AM Peak Hour Intersection Performance

| Intersection | Int Type | LoS | DoS | Ave <br> Delay <br> $(\mathbf{s})$ | Worst <br> Movement <br> delay (s) | Longest <br> Queue (m) | Longest Queue <br> Movement |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northern Rd / <br> Hastings Rd | Priority | $\mathrm{F}^{*}$ | 0.983 | 15.1 | 130.2 | 93 | Hastings Road <br> right turn |
| Old Northern Rd / <br> Gilbert Rd | Signals | C | 0.900 | 31.6 | 49.7 | 182 | Gilbert Road <br> right turn |
| Old Northern Rd / <br> Glenhaven Rd | Priority | F* $^{*}$ | 0.936 | 10.9 | 96.7 | 67 | Glenhaven Road <br> right turn |
| Old Northern Road / <br> Kenthurst Road | Signals | B | 0.782 | 24.5 | 51.3 | 167 | Old Northern Road <br> eastbound thru |
| Old Northern Road / <br> New Line Road | Round | About | A | 0.792 | 12.7 | 17.2 | Old Northern Road <br> eastbound <br> left turn |
| New Line Rd / <br> Hastings Rd | Signals | B | 0.847 | 26.2 | 48.0 | 183 | New Line Road <br> northbound |

Source: Maunsell, 2008
LoS - Level of Service, DoS - Degree of Saturation

* Level of Service for Priority intersections is based on the worst movement average delay.

Level of Service for Signalised intersections is based on average intersection delay.
The SIDRA results for the AM peak suggest that most of the intersections surrounding the study area are operating at/close to capacity ( $\mathrm{DoS}>0.8$ ). The priority intersections at Glenhaven Road and Hastings Road with Old Northern Road operate unsatisfactorily and have significant delays and queuing at the minor approaches as it is very difficult to find a gap to enter into the main stream traffic at Old Northern Road.

At the intersection of Old Northern Road/Gilbert Road, it is observed that large amount of green time has been given to the traffic on Old Northern Road. This has caused significant queuing on the Gilbert Road approach, especially the right turn movement into Old Northern Road.

The intersections of Old Northern Road/Kenthurst Road, Old Northern Road/New Line Road and New Line Road/Hastings Road operate with small amount of spare capacity.

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Table 2.8: PM Peak Hour Intersection Performance Assessment

| Intersection | Int Type | LoS | DoS | Ave Delay (s) | Worst Movement delay (s) | Longest Queue (m) | Longest Queue Movement |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northern Rd / Hastings Rd | Priority | F* | 0.956 | 11.9 | 119.2 | 72 | Hastings Road right turn |
| Old Northern Rd/ Gilbert Rd | Signals | C | 0.867 | 31.3 | 52.1 | 184 | Old Northern Road northbound |
| Old Northern Rd / <br> Glenhaven Rd | Priority | D* | 0.824 | 9.4 | 47.6 | 62 | Old Northern Road southbound right turn |
| Old Northern Road / Kenthurst Road | Signals | B | 0.882 | 27.7 | 43.7 | 154 | Old Northern Road westbound right turn |
| Old Northern Road / New Line Road | Round about | A | 0.467 | 9.8 | 14.9 | 26 | New Line Road northbound |
| New Line Rd/ <br> Hastings Rd | Signals | C | 0.909 | 32.1 | 55.3 | 207 | New Line Road northbound |

Source: Maunsell, 2008
LoS - Level of Service, DoS - Degree of Saturation

* Level of Service for Priority intersections is based on the worst movement average delay.

Level of Service for Signalised intersections is based on average intersection delay.
The SIDRA results for the PM peak suggest that all the intersections are operating at/close to capacity (DoS > 0.8), except the roundabout at Old Northern Road/New Line Road. The priority intersections are not performing satisfactorily due to delays experienced on the minor approaches. Significant queuing is observed on the northbound direction at other intersections at Old Northern Road and New Line Road with heavy traffic movements returning home in this direction.

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### 3.0 Future Transport Conditions

This section reviews the likely impacts of changes to the traffic flows and the road network for the future assessment year before any proposed development occurs in South Dural. Future year assessment will be undertaken for 2018 (10 years from existing) when it is anticipated that the development would be completed and fully occupied by this time.

### 3.1 Background Traffic Growth

A growth rate of $1.68 \%$ per annum has been determined based on historical RTA Traffic Volume Data. This yearly growth rate has been applied to the existing network flows to determine the future traffic conditions in 2018 (without the proposed development). There are no significant development sites within the immediate vicinity of the site. Therefore, it is assumed that this background growth rate will cover any traffic generation associated with other proposed local in-fill developments. The estimated traffic volumes at key intersections under this scenario (2018 without development) are presented in Appendix C.

### 3.2 Mitigation Measures

To cater for the background traffic growth in the vicinity of the study area, local infrastructure upgrades are required for the road network to continue to operate effectively.

Old Northern Road and New Line Road are currently approaching capacity during the peak hours. With the background traffic growth, it is assumed that both roads in the vicinity of the study area will need to be duplicated before 2018.

It is understood that the RTA is planning for the signalisation of intersection at Old Northern Road/Hastings Road, due to its current poor level of service. Therefore, it is assumed that this intersection will be upgraded to traffic signals with additional turning lanes for any future year assessment. The proposed layout of this intersection is shown in Figure 3.1.

With the duplication of Old Northern Road, more delays are expected for traffic from Glenhaven Road to be able to find a gap to enter the main road. To maintain a reasonable level of service for this intersection while maintaining all traffic movements, it is proposed that this intersection will be signalised with additional turning lanes when Old Northern Road is duplicated. The proposed layout of this intersection is shown in Figure 3.2.

Figure 3.1: Proposed Layout of Old Northern Road/Hastings Road (2018 Base)


Source: Maunsell, 2008

Figure 3.2: Proposed Layout of Old Northern Road/Glenhaven Road (2018 Base)


Source: Maunsell, 2008

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Apart from the above major intersection upgrades, a short left turn lane of 100 m is required on the western approach of the Old Northern Road/New Line Road and on the southern approach of Old Northern Road/Gilbert Road to maintain reasonable level of service in the future year. Intersection layouts for the 2018 without development scenario are included as Appendix C.

### 3.3 Intersection Assessment

The intersections within the study area with the proposed upgrades discussed in Section 3.3 have been assessed using SIDRA Intersection 3.2. The results are presented in Table 3.1 and Table 3.2 for the future year scenario. Detailed results are provided in Appendix D.

Table 3.1: 2018 AM Peak Intersection Performance - without proposed development

| Intersection | Int Type | LoS | Dos | Ave <br> Delay <br> $(\mathbf{s})$ | Worst <br> Movement <br> delay (s) | Longest <br> Queue (m) | Longest Queue <br> Movement |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northern Rd / <br> Hastings Rd | Signals | B | 0.701 | 23.6 | 44.3 | 129 | Hastings Road <br> left turn |
| Old Northern Rd / <br> Gilbert Rd | Signals | C | 0.717 | 29.3 | 42.3 | 129 | Gilbert Road <br> right turn |
| Old Northern Rd / <br> Glenhaven Rd | Signals | B | 0.630 | 19.0 | 42.2 | 101 | Old Northern Road <br> northbound thru |
| Old Northern Road / <br> Kenthurst Road | Signals | B | 0.807 | 24.8 | 40.7 | Old Northern Road <br> westbound <br> right turn |  |
| Old Northern Road / <br> New Line Road | Round <br> about | B | 0.968 | 24.6 | 54.0 | 248 | New Line Road <br> northbound |
| New Line Rd / <br> Hastings Rd | Signals | B | 0.751 | 20.0 | 49.5 | 132 | New Line Road <br> northbound |

Source: Maunsell, 2008
LoS - Level of Service, DoS - Degree of Saturation
Level of Service for Signalised intersections is based on average intersection delay.

Table 3.2: 2018 PM Peak Intersection Performance - without proposed development

| Intersection | Int Type | LoS | DoS | Ave <br> Delay <br> $(\mathbf{s})$ | Worst <br> Movement <br> delay (s) | Longest <br> Queue (m) | Longest Queue <br> Movement |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northern Rd / <br> Hastings Rd | Signals | B | 0.848 | 22.9 | 42.2 | 127 | Old Northern Road <br> southbound <br> left turn |
| Old Northern Rd / <br> Gilbert Rd | Signals | B | 0.700 | 27.7 | 49.6 | 126 | Old Northern Road <br> northbound |
| Old Northern Rd / <br> Glenhaven Rd | Signals | B | 0.836 | 27.4 | 47.5 | 150 | Old Northern Road <br> southbound <br> right turn |
| Old Northern Road / <br> Kenthurst Road | Signals | C | 0.910 | 34.5 | 60.1 | 227 | Old Northern Road <br> westbound |
| right turn |  |  |  |  |  |  |  |

Source: Maunsell, 2008
LoS - Level of Service, DoS - Degree of Saturation
Level of Service for Signalised intersections is based on average intersection delay.

## D R A F T

The SIDRA results show that all intersections with the proposed upgrades will be operating with small amount of spare capacity and acceptable level of service in the future year scenario during both peak hour periods.

## D R A F T

### 4.0 Proposed Development

The proposed development at South Dural is estimated to yield approximately 2,940 dwellings ranging from residential lots of various densities to integrated housing. The dwelling type and the number of lots of each type of housing are shown in Table 4.1. The estimates of residential dwellings have been prepared by Inspire Urban Design \& planning.

Table 4.1: Proposed Dwelling Type and Number

| Dwelling Type | Number of Dwellings <br> (approximately) |
| :---: | :---: |
| Large Lots | 100 |
| Conventional Lots | 800 |
| Cottage Lots | 240 |
| Integrated Housing | 1,800 |

Source: Inspire Urban Design and Planning, 2008
The total area of retail space within the proposed development is estimated to be approximately $1,000 \mathrm{~m}^{2}$. The proposed master plan for South Dural is shown in Figure 4.1.

Figure 4.1: Proposed Master Plan Layout


[^1]
## D R A F T

### 5.0 Access Strategy

An access strategy for the site has been developed to determine the number of accesses the site requires to accommodate for the development traffic. This has been based on the number of trips expected to be generated by the development, the distribution of these trips to the surrounding areas and the internal road layout within the development.

### 5.1 Site Access

The site will generate approximately 1,900 vehicle movements in each peak hour. This would require 6 access locations to effectively manage traffic movements and to ensure that the environmental capacity levels for residential roads (approximately 300 vehicles per hour) are not exceeded. Figure 5.1 shows the indicative location of proposed accesses and the lot catchment area for each access.

Figure 5.1: Access Strategy Plan


Source: Maunsell, 2008

## D R A F T

Proposed access points to/from the development are situated at the following locations:

- Old Northern Road - at existing signalised intersection opposite Gilbert Road (Access 1);
- Old Northern Road - at proposed signalised intersection opposite Glenhaven Road (Access 2);
- Old Northern Road - new signalised access south of Malabar Road (Access 3);
- Old Northern Road - new signalised access north of Malabar Road (Access 4);
- New Line Road - at existing roundabout with access to Bunnings (Access 5); and
- New Line Road - at proposed roundabout with Sebastian Drive (Access 6).

Based on Journey to Work Data 2006, the majority of trips (64\%) from the study area are assumed to travel south and east towards Sydney, Parramatta, Ryde and Blacktown. This means that the majority of trips could potentially enter and leave the development via Access 1.

To alleviate the amount of vehicles using Access 1 and to reduce the amount of vehicles travelling through the development, three additional accesses are proposed on Old Northern Road. These additional access locations are proposed at the intersection with Glenhaven Road, south of Malabar Road as well as north of Malabar Road (Accesses 2, 3 and 4). These accesses are also expected to accommodate vehicles travelling between the development and Round Corner as well as Baulkham Hills.

Two other accesses are proposed on New Line Road (Accesses 5 and 6) to accommodate for vehicle movements generated by the eastern side of the development.

All accesses should be designed or upgraded to provide safe access and egress for emergency vehicles to access the proposed development in the event of a bush fire.

### 5.2 Internal Road Network

The proposed master plan for South Dural shows the internal connecting roads between the various parts of the proposed development. It will be important to design the internal road layout to minimise opportunities for rat-running particularly for traffic using the internal collector road system as an alternative to avoid the heavy traffic flows on the external road network. The limited capacity at the intersection of Old Northern Road/Gilbert Road (Access 1), in particular the turning lanes in and out of the study area has been designed to avoid all southbound traffic during the AM peak using the internal road and this access as their preferred route and vice versa.

All internal roads should be designed or upgraded to provide safe access and egress for emergency vehicles to access the proposed development in the event of a bush fire. Perimeter roads following the proposed riparian corridor with appropriate setback zones should be allowed to promote effective bush fire management.

All the site access roads will connect to a main internal collector road as shown in Figure 5.1. This collector road will link neighbourhoods, local centre and the local streets within the proposed development.

### 5.3 Pedestrians and Cyclists

Internal cycle and pedestrian (shared) paths will be connected to existing shared path on Old Northern Road. The proposed traffic signals at Glenhaven Road (Access 2), Access 3 and Access 4 will improve pedestrian/cyclist connection between the study area and Round Corner.

Internal cycle and pedestrian paths will also be connected to existing footpaths and on-road cycle network on New Line Road. Footpaths may need to be provided at certain sections of roads surrounding the study area, especially from the access locations to bus stops.

## D R A F T

### 5.4 Public Transport

The majority of the study area will be within the 400 m catchment of current bus routes operating on Old Northern Road, New Line Road and Hastings Road. Therefore, most of the residents will be within a 400 m safe walking distance of an existing or potential bus route operating on the external road network.

The proposed development may support the provision of improved service frequencies. There is potential to serve the site directly by diverting an existing bus route on Old Northern Road to operate along the internal collector road, if demand is sufficient when the whole site is completed. This proposal will be discussed with the bus operator and MoT.

If a bus route is to be operated along the internal collector road and some of the site accesses, these roads will need to be designed to accommodate bus movements.

### 6.0 Traffic Impact Assessment

This section assesses the likely traffic impacts of the proposed development on the local road network and recommends mitigation measures to alleviate any impacts.

### 6.1 Traffic Generation

The RTA Guide to Traffic Generating Developments (2002) has been used to determine the number of vehicle trips the development will generate.

It has been assumed that the lifestyle lots, conventional lots and cottage lots are of low density and the integrated housing is of medium density. Therefore the following peak hour (AM and PM peak) trip rates have been used:

- Low density residential - 0.85 trips per dwelling; and
- Medium density residential -0.5 trips per dwelling.

Using these trip rates, the proposed development in South Dural will generate a total of 1,870 vehicle movements during each of the peak hours. It has been assumed that during the AM peak $80 \%$ of trips will be leaving the site and $20 \%$ will enter the site, due to the residential nature of the development. This distribution is reversed during the PM peak.

For the purpose of this analysis it has been assumed that trips generated by the local centre within the development will be self-contained and therefore have not been included in the assessment of the external road network.

### 6.2 Traffic Distribution and Assignment

Trip distribution and assignment for the development trips has been determined based on existing Journey to Work pattern, as described in Section 2.2. The traffic pattern of both Baulkham Hills and Hornsby LGAs has been taken into account of determining the trip distribution pattern of the proposed development. Table 6.1 shows the destinations/travel directions and the proportion of development trips that will be travelling to.

Table 6.1: Distribution of Proposed Development Trips

| Destination | \% of trips |
| :--- | :--- |
| Baulkham Hills | $19 \%$ |
| Hornsby | $17 \%$ |
| South | $56 \%$ |
| East | $5 \%$ |
| West | $2 \%$ |

Source: Journey to Work (2006)
The total amount of development trips have been split between the six catchment areas of the proposed development, as seen in Figure 5.1. It has been assumed that vehicles will, in general, use the access associated with the catchment area. The distribution of trips in the AM peak has been reversed in the PM peak.

Development trips have been added to the 2018 base network according to the distribution pattern as illustrated in Table 6.1. The turning flows for all intersections are included as Appendix E.

## D R A F T

### 6.3 Intersection Assessment

All the proposed access points and key intersections surrounding the study area have been modelled in SIDRA Intersection 3.2. The intersection performance results are shown in the following tables (Table 6.2 and Table 6.3). Detailed results are provided in Appendix F.

Table 6.2: 2018 AM Peak Intersection Performance - with proposed development

| Intersection | Int Type | LoS | DoS | Ave Delay (s) | Worst Movement delay (s) | Longest Queue (m) | Longest Queue Movement |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northern Rd / Hastings Rd | Signals | C | 0.926 | 39.3 | 61.1 | 289 | Hastings Road left turn |
| Old Northern Rd / <br> Gilbert Rd / Access 1 | Signals | D | 0.869 | 44.4 | 71.0 | 166 | Old Northern Road southbound |
| Old Northern Rd / <br> Glenhaven Rd / Access 2 | Signals | C | 0.773 | 34.6 | 56.6 | 272 | Old Northern Road southbound |
| Old Northern Road / Access 3 | Signals | A | 0.536 | 11.0 | 47.4 | 119 | Old Northern Road northbound |
| Old Northern Road / Access 4 | Signals | B | 0.801 | 24.3 | 38.4 | 176 | Old Northern Road northbound |
| Old Northern Road / <br> Kenthurst Road | Signals | B | 0.840 | 27.0 | 51.1 | 160 | Old Northern Road westbound right turn |
| Old Northern Road / <br> New Line Road | Round about | E | 1.105 | 68.0 | 226.5 | 905 | New Line Road northbound |
|  | Signals | D | 0.950 | 44.8 | 78.9 | 242 | Old Northern Road southbound right turn |
| New Line Road / Access to Bunnings / Access 5 | Round about | A | 0.540 | 6.9 | 15.4 | 48 | New Line Road northbound |
| New Line Road / Sebastian Drive / Access 6 | Signals | B | 0.735 | 16.2 | 35.3 | 130 | New Line Road northbound |
| New Line Rd/ Hastings Rd | Signals | B | 0.867 | 24.2 | 51.5 | 169 | New Line Road northbound |

Source: Maunsell, 2008
LoS - Level of Service, DoS - Degree of Saturation
Level of Service for Signalised intersections is based on average intersection delay.

Table 6.3: 2018 PM Peak Intersection Performance - with proposed development

| Intersection | Int Type | LoS | DoS | Ave <br> Delay <br> (s) | Worst <br> Movement <br> delay (s) | Longest <br> Queue (m) | Longest Queue <br> Movement |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northern Rd / <br> Hastings Rd | Signals | C | 0.998 | 30.8 | 87.5 | 227 | Old Northern Road <br> northbound <br> right turn |
| Old Northern Rd / <br> Gilbert Rd / Access 1 | Signals | D | 0.834 | 50.8 | 75.0 | 211 | Old Northern Road <br> northbound |
| Old Northern Rd / <br> Glenhaven Rd / Access 2 | Signals | C | 0.887 | 40.9 | 67.0 | 240 | Old Northern Road <br> southbound right <br> turn |


| Intersection | Int Type | LoS | Dos | Ave <br> Delay <br> (s) | Worst <br> Movement <br> delay (s) | Longest <br> Queue (m) | Longest Queue <br> Movement |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northern Road / <br> Access 3 | Signals | A | 0.530 | 14.5 | 53.8 | 126 | Old Northern Road <br> southbound |
| Old Northern Road / <br> Access 4 | Signals | A | 0.597 | 13.1 | 69.7 | 176 | Old Northern Road <br> southbound |
| Old Northern Road / <br> Kenthurst Road | Signals | E | 1.000 | 57.8 | 120.1 | 449 | Kenthurst Road <br> right turn |
| Old Northern Road / <br> New Line Road | Round <br> about | A | 0.819 | 13.4 | 21.0 | 97 | New Line Road <br> northbound |
|  | Signals | C | 0.846 | 33.5 | 47.4 | 126 | Old Northern Road <br> southbound <br> right turn |
| New Line Road / Access <br> to Bunnings / Access 5 | Round <br> about | A | 0.760 | 7.0 | 16.7 | 110 | New Line Road <br> northbound |
| New Line Road / <br> Sebastian Drive / Access <br> 6 | Signals | A | 0.776 | 12.3 | 46.9 | 212 | New Line Road <br> northbound |
| New Line Rd / Hastings <br> Rd | Signals | C | 0.911 | 37.6 | 81.4 | 338 | New Line Road <br> northbound |

Source: Maunsell, 2008
LoS - Level of Service, DoS - Degree of Saturation

* Level of Service for Priority intersections is based on the worst movement average delay.

Level of Service for Signalised intersections is based on average intersection delay.
The SIDRA analysis shows that intersections of Old Northern Road/Hastings Road and New Line Road/Hastings Road will both operate satisfactorily with the additional development traffic on the road network during both peak periods. Queueing at both intersections has increased when compared to the 2018 base case.

The intersection of Old Northern Road/New Line Road has been assessed as a roundabout (same geometry as in 2018 base case). However, the intersection will not operate satisfactorily ( $\mathrm{DoS}>1$ ) with extensive queueing on New Line Road during the AM peak. Therefore, the intersection has also been assessed as traffic signals and it operates satisfactorily with improved level of service during both peak hours. The proposed layout of the signalised intersection is shown in Figure 6.1.

Figure 6.1: Potential Traffic Signals Layout at Old Northern Road/New Line Road


[^2]
## D R A F T

All the intersections associated with the proposed access locations will operate satisfactorily during the AM and PM peak periods in 2018, by introducing the access roads and additional turning lanes leading into the proposed development. Lengthening of the right turn lane onto Glenhaven Road to 150 m is required to accommodate the high amount of right turning traffic in the PM peak period.

The intersection of Old Northern Road/Kenthurst Road operates adequately in the AM peak period however it operates at capacity in the PM peak period due to a heavy right turn from Old Northern Road to Kenthurst Road. The heavy right turn is not attributable to development traffic as it is evident in 2018 base case scenario.

At this stage, it is proposed that all movements are allowed at all the accesses, except a right turn ban is proposed from Old Northern Road into Access 2 opposite Glenhaven Road.

All proposed intersection layouts for 2018 (with development scenario) are included as Appendix E.

### 6.4 Infrastructure Funding

The costs of proposed infrastructure upgrades will be fully or partially contributed by the developers of South Dural in order to alleviate the impacts generated by their development. If the proposed upgrades offer sole benefits to the development (access roads and internal roads), then they would be fully funded by the developer.

If the upgrades satisfy not only the demand of the new development, but also some regional traffic demand or to make up some existing deficiency, then only a portion of the cost for the proposed upgrade will be apportioned to the developer.

Upon agreement of the proposed infrastructure package and an associated scope of works for each measure, a more through scrutiny of likely costs can be undertaken if appropriate within the context of the planning process.

## D R A F T

### 7.0 Conclusion

Maunsell has prepared a TIA for the proposed development of South Dural in Sydney's north west. The proposed development includes approximately 2,940 residential dwellings and a small local centre at the southern edge of the proposed development. The proposed development in South Dural is not considered to have an adverse traffic impact on the surrounding road network.

### 7.1 Access Strategy

The development will require six access points on to the local road network. The number of accesses has been determined through the amount of trips the development is expected to generate and the environmental capacity of the access roads.

Four signalised access points will be provided along Old Northern Road opposite Gilbert Road, Glenhaven Road, south of Malabar Road and south of Kenthurst Road. These signalised intersections will formalise pedestrian crossing movements across Old Northern Road.

Two access points will also be provided on New Line Road to cater for the eastern side of the development. The proposed access opposite Bunnings will be controlled by a roundabout, while the other proposed access opposite Sebastian Drive will be controlled by traffic signals.

The internal road network will consist of a main collector road that runs through the site, connecting the neighbourhoods, the local centre and local streets within the development.

All internal roads should be designed or upgraded to provide safe access and egress for emergency vehicles to access the proposed development in the event of a bush fire. Perimeter roads following the proposed riparian corridor with appropriate setback zones should be allowed to promote effective bush fire management.

### 7.2 Intersection Performance

It is determined, as part of this study that road network upgrades are required to cater for the background traffic growth in the future years before the development of South Dural. These upgrades include:

- Duplication of Old Northern Road between Hastings Road and New Line Road;
- Duplication of New Line Road between Old Northern Road and Hastings Road;
- Signalisation of Old Northern Road/Hastings Road;
- Signalisation of Old Northern Road/Glenhaven Road;
- Addition of short (100m) left turn lane on the western approach of the Old Northern Road/New Line Road; and
- Addition of short (100m) left turn lane on the southern approach of Old Northern Road/Gilbert Road.

The infrastructure upgrades will provide additional capacity such that the road network can continue to operate effectively during both peak periods in 2018 before South Dural development occurs.

All intersections within the study area have been modelled with additional development trips and access roads and turning lanes leading to the proposed development. The SIDRA analyses confirm that all intersections and accesses will operate at appropriate level of service, with inherent reserve capacity.

## D R A F T

### 7.3 Transport and Accessibility

Internal cycle and pedestrian (shared) paths will be connected to existing shared path on Old Northern Road. The proposed traffic signals at Glenhaven Road (Access 2), Access 3 and Access 4 will improve pedestrian/cyclist connection between the site and Round Corner.

Internal cycle and pedestrian paths will also be connected to existing footpaths and on-road cycle network on New Line Road. Footpaths may need to be provided at certain sections of roads surrounding the study area, especially from the access locations to bus stops.

The majority of the study area will be within the 400 m catchment of current bus routes operating on Old Northern Road, New Line Road and Hastings Road. Therefore, most of the residents will be within a 400 m safe walking distance of an existing or potential bus route operating on the external road network.

The proposed development may support the provision of improved service frequencies. There is potential to serve the site directly by diverting an existing bus route on Old Northern Road to operate along the internal collector road, if demand is sufficient when the whole site is completed. This proposal will be discussed with the bus operator and MoT.

If a bus route is to be operated along the internal collector road and some of the site accesses, these roads will need to be designed to accommodate bus movements.

D R A F T

## Appendix A Surveyed Traffic Data and Existing Traffic Flows

D R A F T
2008 AM Peak Turning Flows

| Old Northern Road / Glenhaven Road | Old Northern Road / Gilbert Road | Old Northern Road Hastings Road |
| :---: | :---: | :---: |
|  |  |  |
| Old Northern Road / New Line Road | Old Northern Road / Kenthurst Road | New Line Road / Hastings Road |
|  |  |  |

[^3]D R A F T
2008 PM Peak Turning Flows

| Old Northern Road / Glenhaven Road | Old Northern Road / Gilbert Road | Old Northern Road Hastings Road |
| :---: | :---: | :---: |
|  |  |  |
| Old Northern Road / New Line Road | Old Northern Road / Kenthurst Road | New Line Road / Hastings Road |
|  |  |  |

[^4]D R A F T

Appendix B 2008 SIDRA Results

## Movement Summary

## Glenhaven Road / Old Northern Road

## 2008 AM peak

## Give-way

Vehicle Movements


Symbols which may appear in this table:
Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: 2008 AM (base)
$\mathrm{K}: \backslash 60044833$ South_Dural $\backslash 4$. Tech work area $\backslash 4.3$. Engineering $\backslash T r a f f i c \backslash S I D R A \backslash w i t h ~ d e v ~ t r a f f i c \backslash E \_G l e n h a v e n ~ R d \_O l d ~$
Northern Rd_v2_with dev_v2.aap
Processed Aug 19, 2008 04:03:00PM
A0194, Maunsell Australia Pty Ltd, Large Office
Produced by SIDRA Intersection 3.2.0.1455

## Movement Summary

## Glenhaven Road / Old Northern Road

## 2008 PM peak

## Give-way

Vehicle Movements


Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: 2008 PM (base)
$\mathrm{K}: \backslash 60044833$ South_Dural $\backslash 4$. Tech work area $\backslash 4.3$. Engineering $\backslash T r a f f i c \backslash S I D R A \backslash w i t h ~ d e v ~ t r a f f i c \backslash E \_G l e n h a v e n ~ R d \_O l d ~$
Northern Rd_v2_with dev_v2.aap
Processed Aug 19, 2008 01:37:09PM

A0194, Maunsell Australia Pty Ltd, Large Office
Produced by SIDRA Intersection 3.2.0.1455

## Movement Summary

## Gilbert Road / Old Northern Road

## 2008 AM peak

```
Signalised - Fixed time Cycle Time = 70 seconds
```

Vehicle Movements


## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> $(\mathbf{m})$ | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P13 | 50 | 22.4 | LOS C | 0 | 0.80 | 0.80 |
| P15 | 50 | 26.6 | LOS C | 0 | 0.87 | 0.87 |
| All Peds | $\mathbf{1 0 0}$ | $\mathbf{2 4 . 5}$ | LOS B | $\mathbf{0}$ | $\mathbf{0 . 8 4}$ | $\mathbf{0 . 8 4}$ |

Symbols which may appear in this table:
Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity


## Following LOS

\# - Based on density for continuous movements

## Movement Summary

## Gilbert Road / Old Northern Road

## 2008 PM peak

Signalised - Fixed time Cycle Time $\mathbf{= 8 0}$ seconds

Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northern Road (south east) |  |  |  |  |  |  |  |  |  |  |
| 21 | L | 301 | 1.0 | 0.614 | 28.9 | LOS C | 110 | 0.85 | 0.84 | 33.4 |
| 22 | T | 737 | 2.4 | 0.867 | 32.3 | LOS C | 184 | 0.97 | 0.99 | 31.8 |
| Approach |  | 1038 | 2.0 | 0.867 | 31.3 | LOS C | 184 | 0.93 | 0.95 | 32.2 |
| Old Northern Road (north west) |  |  |  |  |  |  |  |  |  |  |
| 28 | T | 376 | 6.4 | 0.222 | 10.2 | LOS A | 45 | 0.55 | 0.46 | 46.9 |
| $29$ | R | 169 | 1.2 | 0.816 | 52.1 | LOS D | 64 | 1.00 | 0.96 | 24.6 |
| Approach |  | 545 | 4.8 | 0.816 | 23.2 | LOS B | 64 | 0.69 | 0.61 | 36.6 |
| Gilbert Road |  |  |  |  |  |  |  |  |  |  |
| 30 | L | 217 | 0.9 | 0.235 | 20.4 | LOS B | 45 | 0.60 | 0.77 | 38.4 |
| 32 | R | 497 | 0.8 | 0.861 | 45.1 | LOS D | 157 | 1.00 | 1.05 | 26.7 |
| Approach |  | 714 | 0.8 | 0.861 | 37.6 | LOS C | 157 | 0.88 | 0.96 | 29.5 |
| All Vehicles |  | 2297 | 2.3 | 0.867 | 31.3 | LOS C | 184 | 0.86 | 0.87 | 32.2 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> $(\mathbf{m})$ | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P13 | 50 | 27.2 | LOS C | 0 | 0.82 | 0.82 |
| P15 | 50 | 22.5 | LOS C | 0 | 0.75 | 0.75 |
| All Peds | $\mathbf{1 0 0}$ | $\mathbf{2 4 . 9}$ | LOS B | $\mathbf{0}$ | $\mathbf{0 . 7 9}$ | $\mathbf{0 . 7 9}$ |

Symbols which may appear in this table:
Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity


## Following LOS

\# - Based on density for continuous movements

## Movement Summary

## Hastings Road / Old Northern Road

## 2008 AM peak

## Give-way

Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northern Rd (SE) |  |  |  |  |  |  |  |  |  |  |
| 22 | T | 464 | 12.9 | 0.258 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 60.0 |
| 23 | R | 206 | 2.4 | 0.601 | 27.4 | LOS B | 28 | 0.91 | 1.13 | 34.2 |
| Approach |  | 670 | 9.7 | 0.601 | 8.4 | LOS A | 28 | 0.28 | 0.35 | 48.7 |
| Hastings Road |  |  |  |  |  |  |  |  |  |  |
| 24 | L | 459 | 0.7 | 0.591 | 13.7 | LOS A | 25 | 0.76 | 1.06 | 43.5 |
| 26 | R | 170 | 1.2 | 0.983 | 130.2 | LOS F | 93 | 1.00 | 1.96 | 13.0 |
| Approach |  | 629 | 0.8 | 0.981 | 45.2 | LOS D | 93 | 0.83 | 1.31 | 26.7 |
| Old Northern Rd (NW) |  |  |  |  |  |  |  |  |  |  |
| 27 | L | 364 | 0.8 | 0.619 | 8.2 | LOS A | 0 | 0.00 | 0.67 | 49.0 |
| 28 | T | 797 | 4.9 | 0.619 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 60.0 |
| Approach |  | 1161 | 3.6 | 0.619 | 2.6 | LOS A |  | 0.00 | 0.21 | 56.0 |
| All Vehicles |  | 2460 | 4.6 | 0.983 | 15.1 | $\begin{gathered} \text { Applicablet } \end{gathered}$ | 93 | 0.29 | 0.53 | 42.4 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: 2008 AM (base)
K: \60044833_South_Dural\4. Tech work area\4.3. Engineering $\backslash$ Traffic $\backslash$ SIDRA $\backslash$ with dev traffic $\backslash$ Hastings Rd_Old Northern Rd_v2_with dev.aap
Processed Aug 19, 2008 03:30:48PM

A0194, Maunsell Australia Pty Ltd, Large Office
Produced by SIDRA Intersection 3.2.0.1455

## Movement Summary

## Hastings Road / Old Northern Road

## 2008 PM peak

## Give-way

Vehicle Movements

| Mov ID Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northern Rd (SE) |  |  |  |  |  |  |  |  |  |
| 22 T | 873 | 1.7 | 0.453 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 60.0 |
| 23 R | 270 | 0.4 | 0.447 | 16.6 | LOS B | 22 | 0.78 | 1.03 | 41.1 |
| Approach | 1143 | 1.4 | 0.453 | 3.9 | LOS A | 22 | 0.18 | 0.24 | 54.1 |
| Hastings Road |  |  |  |  |  |  |  |  |  |
| 24 L | 393 | 0.3 | 0.343 | 10.2 | LOS A | 12 | 0.49 | 0.86 | 46.8 |
| 26 R | 152 | 0.7 | 0.956 | 119.2 | LOS F | 72 | 0.99 | 1.72 | 13.9 |
| Approach | 545 | 0.4 | 0.954 | 40.6 | LOS C | 72 | 0.63 | 1.10 | 28.3 |
| Old Northern Rd (NW) |  |  |  |  |  |  |  |  |  |
| 27 L | 380 | 0.3 | 0.434 | 8.2 | LOS A | 0 | 0.00 | 0.67 | 49.0 |
| 28 T | 442 | 1.8 | 0.434 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 60.0 |
| Approach | 822 | 1.1 | 0.434 | 3.8 | LOS A |  | 0.00 | 0.31 | 54.3 |
| All Vehicles | 2510 | 1.1 | 0.956 | 11.9 | Not <br> Applicable | 72 | 0.22 | 0.45 | 45.2 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: 2008 PM (base)
K: \60044833_South_Dural\4. Tech work area\4.3. Engineering $\backslash$ Traffic $\backslash$ SIDRA $\backslash$ with dev traffic $\backslash$ Hastings Rd_Old Northern Rd_v2_with dev.aap
Processed Aug 19, 2008 03:30:48PM

A0194, Maunsell Australia Pty Ltd, Large Office
Produced by SIDRA Intersection 3.2.0.1455

## Movement Summary

## Old Northern Rd / New Line Rd

## 2008 AM peak

## Roundabout

Vehicle Movements


Symbols which may appear in this table:
Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity


## Following LOS

\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

## Movement Summary

## Old Northern Rd / New Line Rd

## 2008 PM peak

## Roundabout

Vehicle Movements


Symbols which may appear in this table:
Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity


## Following LOS

\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

## Movement Summary

## Kenthurst Rd / Old Northern Rd

## 2008 AM peak

Signalised - Fixed time $\quad$ Cycle Time $=\mathbf{9 0}$ seconds

Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver <br> Delay <br> (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northern Rd east |  |  |  |  |  |  |  |  |  |  |
| 5 | T | 504 | 6.0 | 0.378 | 5.4 | LOS A | 76 | 0.43 | 0.38 | 52.2 |
| 6 | R | 380 | 6.1 | 0.769 | 43.8 | LOS D | 129 | 0.98 | 0.92 | 27.3 |
| Approach |  | 884 | 6.0 | 0.768 | 21.9 | LOS B | 129 | 0.66 | 0.61 | 37.5 |
| Kenthurst Rd |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 502 | 6.0 | 0.541 | 15.7 | LOS B | 86 | 0.67 | 0.79 | 42.2 |
| 9 | R | 206 | 5.8 | 0.743 | 51.3 | LOS D | 81 | 1.00 | 0.90 | 24.9 |
| Approach |  | 708 | 5.9 | 0.743 | 26.1 | LOS B | 86 | 0.76 | 0.82 | 35.2 |
| Old Northern Rd west |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 234 | 6.0 | 0.266 | 17.7 | LOS B | 47 | 0.50 | 0.76 | 40.5 |
| 11 | T | 538 | 5.9 | 0.782 | 29.8 | LOS C | 167 | 0.95 | 0.89 | 33.0 |
| Approach |  | 772 | 6.0 | 0.782 | 26.2 | LOS B | 167 | 0.82 | 0.85 | 34.9 |
| All Vehicles |  | 2364 | 6.0 | 0.782 | 24.5 | LOS B | 167 | 0.74 | 0.75 | 35.9 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Quened | Eff. Stop <br> Rate |
| ---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P3 | 50 | 39.2 | LOS D | 0 | 0.93 | 0.93 |
| P5 | 50 | 20.0 | LOS B | 0 | 0.67 | 0.67 |
| All Peds | $\mathbf{1 0 0}$ | $\mathbf{2 9 . 6}$ | LOS C | $\mathbf{0}$ | $\mathbf{0 . 8 0}$ | $\mathbf{0 . 8 0}$ |

[^5]Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity


## Following LOS

\# - Based on density for continuous movements

## Movement Summary

## Kenthurst Rd / Old Northern Rd

## 2008 PM peak

Signalised - Fixed time Cycle Time $=\mathbf{7 0}$ seconds
Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver <br> Delay <br> (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northern Rd east |  |  |  |  |  |  |  |  |  |  |
| 5 | T | 586 | 6.0 | 0.533 | 9.4 | LOS A | 99 | 0.65 | 0.58 | 47.7 |
| 6 | R | 523 | 5.9 | 0.856 | 40.2 | LOS C | 154 | 1.00 | 1.06 | 28.5 |
| Approach |  | 1109 | 6.0 | 0.856 | 23.9 | LOS B | 154 | 0.81 | 0.80 | 36.2 |
| Kenthurst Rd |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 209 | 6.2 | 0.183 | 10.8 | LOS A | 19 | 0.44 | 0.70 | 46.6 |
| 9 | R | 366 | 6.0 | 0.846 | 43.7 | LOS D | 114 | 1.00 | 1.04 | 27.3 |
| Approach |  | 575 | 6.1 | 0.846 | 31.7 | LOS C | 114 | 0.80 | 0.91 | 32.2 |
| Old Northern Rd west |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 223 | 5.8 | 0.257 | 19.8 | LOS B | 44 | 0.62 | 0.78 | 38.9 |
| 11 | T | 260 | 6.2 | 0.882 | 41.7 | LOS C | 90 | 1.00 | 1.09 | 28.0 |
| Approach |  | 483 | 6.0 | 0.882 | 31.6 | LOS C | 90 | 0.83 | 0.94 | 32.1 |
| All Vehicles |  | 2167 | 6.0 | 0.882 | 27.7 | LOS B | 154 | 0.81 | 0.86 | 34.1 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Quened | Eff. Stop <br> Rate |
| ---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P3 | 50 | 26.6 | LOS C | 0 | 0.87 | 0.87 |
| P5 | 50 | 27.5 | LOS C | 0 | 0.89 | 0.89 |
| All Peds | $\mathbf{1 0 0}$ | $\mathbf{2 7 . 0}$ | LOS B | $\mathbf{0}$ | $\mathbf{0 . 8 8}$ | $\mathbf{0 . 8 8}$ |

[^6]Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity


## Following LOS

\# - Based on density for continuous movements

## Movement Summary

## Hastings Road / New Line Road

## 2008 AM peak

Signalised - Fixed time $\quad$ Cycle Time $=\mathbf{8 0}$ seconds
Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| New Line Rd (south) |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 372 | 0.5 | 0.847 | 39.0 | LOS C | 183 | 0.97 | 1.02 | 28.9 |
| 2 | T | 795 | 5.8 | 0.846 | 32.4 | LOS C | 183 | 0.99 | 1.01 | 31.7 |
| Approach |  | 1167 | 4.1 | 0.846 | 34.5 | LOS C | 183 | 0.98 | 1.01 | 30.8 |
| New Line Rd (north) |  |  |  |  |  |  |  |  |  |  |
| 8 | T | 822 | 6.1 | 0.438 | 0.1 | LOS C\# | 14\# | 0.00 | 0.00 | 59.9 |
| 9 | R | 262 | 0.4 | 0.808 | 48.0 | LOS D | 89 | 1.00 | 0.96 | 25.8 |
| Approach |  | 1084 | 4.7 | 0.808 | 11.7 | LOS A | 89 | 0.24 | 0.23 | 45.4 |
| Hastings Rd |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 190 | 1.1 | 0.206 | 20.2 | LOS B | 39 | 0.59 | 0.77 | 38.6 |
| 12 | R | 380 | 1.3 | 0.826 | 45.1 | LOS D | 121 | 1.00 | 0.99 | 26.8 |
| Approach |  | 570 | 1.2 | 0.826 | 36.8 | LOS C | 121 | 0.86 | 0.92 | 29.8 |
| All Vehicles |  | 2821 | 3.8 | 0.847 | 26.2 | LOS B | 183 | 0.67 | 0.69 | 34.9 |

Symbols which may appear in this table:
Following Degree of Saturation
\# x $=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements

Following Queue
\# - Density for continuous movement

## SIDRA SOLUTIONS

Site: 2008 AM (base)
$\mathrm{K}: \backslash 60044833$ _South_Dural\4. Tech work area\4.3. Engineering\Traffic \SIDRA 1 with dev traffic $\backslash$ Hastings Rd_New Line
Rd_v2_with dev.aap
Processed Aug 19, 2008 02:25:30PM
A0194, Maunsell Australia Pty Ltd, Large Office
Produced by SIDRA Intersection 3.2.0.1455
Copyright 2000-2007 Akcelik and Associates Pty Ltd

## Movement Summary

## Hastings Road / New Line Road

## 2008 PM peak

```
Signalised - Fixed time Cycle Time = 80 seconds
```

Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver <br> Delay <br> (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| New Line Rd (south) |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 279 | 0.7 | 0.892 | 46.9 | LOS D | 207 | 1.00 | 1.11 | 26.2 |
| 2 | T | 935 | 3.9 | 0.893 | 39.3 | LOS C | 207 | 1.00 | 1.11 | 28.9 |
| Approach |  | 1214 | 3.1 | 0.893 | 41.0 | LOS C | 207 | 1.00 | 1.11 | 28.2 |
| New Line Rd (north) |  |  |  |  |  |  |  |  |  |  |
| 8 | T | 751 | 2.3 | 0.391 | 0.1 | LOS C\# | 13\# | 0.00 | 0.00 | 59.9 |
| 9 | R | 263 | 0.4 | 0.874 | 53.6 | LOS D | 95 | 1.00 | 1.06 | 24.2 |
| Approach |  | 1014 | 1.8 | 0.874 | 14.0 | LOS A | 95 | 0.26 | 0.27 | 43.3 |
| Hastings Rd |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 229 | 1.3 | 0.249 | 20.5 | LOS B | 47 | 0.61 | 0.77 | 38.4 |
| 12 | R | 441 | 0.7 | 0.909 | 55.3 | LOS D | 157 | 1.00 | 1.16 | 23.8 |
| Approach |  | 670 | 0.9 | 0.909 | 43.4 | LOS D | 157 | 0.87 | 1.03 | 27.3 |
| All Vehicles |  | 2898 | 2.1 | 0.909 | 32.1 | LOS C | 207 | 0.71 | 0.80 | 31.9 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements

Following Queue
\# - Density for continuous movement

## SIDRA SOLUTIONS

Site: 2008 PM (base)
$\mathrm{K}: \backslash 60044833$ _South_Dural\4. Tech work area\4.3. Engineering\Traffic\SIDRA\with dev traffic\Hastings Rd_New Line
Rd_v2_with dev.aap
Processed Aug 19, 2008 02:30:24PM
A0194, Maunsell Australia Pty Ltd, Large Office
Produced by SIDRA Intersection 3.2.0.1455
Copyright 2000-2007 Akcelik and Associates Pty Ltd

D R A F T

Appendix C 2018 (without development) Traffic Flows and Intersection Layouts
D R A F T 2018 AM Peak (without development) Turning Flows

| Old Northern Road / Glenhaven Road | Old Northern Road / Gilbert Road | Old Northern Road Hastings Road |
| :---: | :---: | :---: |
|  |  |  |
| Old Northern Road / New Line Road | Old Northern Road / Kenthurst Road | New Line Road / Hastings Road |
|  |  |  |

[^7]D R A F T
2018 PM Peak (without development) Turning Flows

| Old Northern Road / Glenhaven Road | Old Northern Road / Gilbert Road | Old Northern Road Hastings Road |
| :---: | :---: | :---: |
| Old Northern Rd (north) |  |  |
| Old Northern Road / New Line Road | Old Northern Road / Kenthurst Road | New Line Road / Hastings Road |
|  |  |  |

[^8]D R A F T
2018 intersection layout (without development)

South Dural Urban Release Area
K:I60044833_South_Durall|6. Draft docs\6.1. Reports\South Dural TIA_150109_Rev C.doc
Page C-3
K:I60044833_South_Durall6.
Revision C 15 January 2009

D R A F T

Appendix D 2018 (without development) SIDRA Results

## Movement Summary

## Glenhaven Road / Old Northern Road

## 2018 AM peak

Signalised - Fixed time

$$
\text { Cycle Time }=80 \text { seconds }
$$

Vehicle Movements

| Mov ID Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northen Rd (south) |  |  |  |  |  |  |  |  |  |
| 1 L | 130 | 5.4 | 0.129 | 14.8 | LOS B | 22 | 0.43 | 0.73 | 42.8 |
| 2 T | 693 | 10.7 | 0.608 | 25.6 | LOS B | 101 | 0.90 | 0.77 | 35.2 |
| Approach | 823 | 9.8 | 0.608 | 23.9 | LOS B | 101 | 0.83 | 0.77 | 36.2 |
| Old Northern Rd (north) |  |  |  |  |  |  |  |  |  |
| 8 T | 803 | 6.4 | 0.350 | 8.2 | LOS A | 69 | 0.53 | 0.46 | 49.0 |
| 9 R | 241 | 12.9 | 0.630 | 40.0 | LOS C | 81 | 0.96 | 0.84 | 28.7 |
| Approach | 1044 | 7.9 | 0.630 | 15.5 | LOS B | 81 | 0.63 | 0.55 | 42.1 |
| Glenhaven Rd |  |  |  |  |  |  |  |  |  |
| 10 L | 453 | 5.3 | 0.365 | 8.4 | LOS A | 20 | 0.21 | 0.65 | 48.6 |
| 12 R | 189 | 3.2 | 0.595 | 42.2 | LOS C | 63 | 0.97 | 0.81 | 27.8 |
| Approach | 642 | 4.7 | 0.595 | 18.3 | LOS B | 63 | 0.44 | 0.70 | 39.9 |
| All Vehicles | 2509 | 7.7 | 0.630 | 19.0 | LOS B | 101 | 0.64 | 0.66 | 39.5 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> $(\mathbf{m})$ | Prop. <br> Queued | Eff. Stop <br> Rate |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| P1 | 53 | 32.4 | LOS D | 0 | 0.90 | 0.90 |
| P5 | 53 | 32.4 | LOS D | 0 | 0.90 | 0.90 |
| P7 | 53 | 23.3 | LOS C | 0 | 0.76 | 0.76 |
| All Peds | $\mathbf{1 5 9}$ | $\mathbf{2 9 . 4}$ | LOS C | $\mathbf{0}$ | $\mathbf{0 . 8 5}$ | $\mathbf{0 . 8 5}$ |

[^9][^10]
## Movement Summary

## Glenhaven Road / Old Northern Road

## 2018 PM peak

Signalised - Fixed time $\quad$ Cycle Time $=\mathbf{9 0}$ seconds

Vehicle Movements


## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> $(\mathbf{m})$ | Prop. <br> Queued | Eff. Stop <br> Rate |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| P1 | 53 | 39.2 | LOS D | 0 | 0.93 | 0.93 |
| P5 | 53 | 39.2 | LOS D | 0 | 0.93 | 0.93 |
| P7 | 53 | 31.2 | LOS D | 0 | 0.83 | 0.83 |
| All Peds | $\mathbf{1 5 9}$ | $\mathbf{3 6 . 5}$ | LOS C | $\mathbf{0}$ | $\mathbf{0 . 9 0}$ | $\mathbf{0 . 9 0}$ |

[^11][^12]
## Movement Summary

## Gilbert Road / Old Northern Road

## 2018 AM peak

```
Signalised - Fixed time Cycle Time = 90 seconds
```

Vehicle Movements


## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> $(\mathbf{m})$ | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P13 | 50 | 26.4 | LOS C | 0 | 0.77 | 0.77 |
| P15 | 50 | 34.7 | LOS D | 0 | 0.88 | 0.88 |
| All Peds | $\mathbf{1 0 0}$ | $\mathbf{3 0 . 6}$ | LOS C | $\mathbf{0}$ | $\mathbf{0 . 8 2}$ | $\mathbf{0 . 8 2}$ |

Symbols which may appear in this table:
Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity


## Following LOS

\# - Based on density for continuous movements

## Movement Summary

## Gilbert Road / Old Northern Road

## 2018 PM peak

Signalised - Fixed time Cycle Time $=\mathbf{9 0}$ seconds

Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northern Road (south east) |  |  |  |  |  |  |  |  |  |  |
| 21 | L | 356 | 1.1 | 0.386 | 13.2 | LOS A | 50 | 0.38 | 0.74 | 44.1 |
| 22 | T | 870 | 2.4 | 0.680 | 28.1 | LOS B | 126 | 0.92 | 0.80 | 33.9 |
| Approach |  | 1226 | 2.0 | 0.680 | 23.8 | LOS B | 126 | 0.77 | 0.79 | 36.3 |
| Old Northern Road (north west) |  |  |  |  |  |  |  |  |  |  |
| 28 | T | 444 | 6.3 | 0.213 | 10.7 | LOS A | 47 | 0.54 | 0.45 | 46.3 |
| 29 | R | 199 | 1.0 | 0.694 | 49.6 | LOS D | 74 | 1.00 | 0.86 | 25.4 |
| Approach |  | 643 | 4.7 | 0.694 | 22.7 | LOS B | 74 | 0.68 | 0.58 | 36.9 |
| Gilbert Road |  |  |  |  |  |  |  |  |  |  |
| 30 | L | 256 | 0.8 | 0.700 | 36.3 | LOS C | 111 | 0.92 | 0.95 | 30.0 |
| 32 | R | 587 | 0.9 | 0.700 | 37.6 | LOS C | 120 | 0.93 | 0.89 | 29.5 |
| Approach |  | 843 | 0.8 | 0.700 | 37.2 | LOS C | 120 | 0.93 | 0.91 | 29.7 |
| All Vehicles |  | 2712 | 2.3 | 0.700 | 27.7 | LOS B | 126 | 0.80 | 0.78 | 34.1 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> $(\mathbf{m})$ | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P13 | 50 | 29.6 | LOS C | 0 | 0.81 | 0.81 |
| P15 | 50 | 25.7 | LOS C | 0 | 0.76 | 0.76 |
| All Peds | $\mathbf{1 0 0}$ | $\mathbf{2 7 . 6}$ | LOS B | $\mathbf{0}$ | $\mathbf{0 . 7 8}$ | $\mathbf{0 . 7 8}$ |

Symbols which may appear in this table:
Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity


## Following LOS

\# - Based on density for continuous movements

## Movement Summary

## Hastings Road / Old Northern Road

## 2018 AM peak

Signalised - Fixed time Cycle Time $=\mathbf{8 0}$ seconds

Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northern Rd (SE) |  |  |  |  |  |  |  |  |  |  |
| 22 | T | 548 | 13.0 | 0.244 | 7.1 | LOS A | 48 | 0.47 | 0.40 | 50.2 |
| 23 | R | 243 | 2.5 | 0.633 | 35.4 | LOS C | 72 | 0.97 | 0.91 | 30.4 |
| Approach |  | 791 | 9.7 | 0.633 | 15.8 | LOS B | 72 | 0.62 | 0.56 | 41.9 |
| Hastings Road |  |  |  |  |  |  |  |  |  |  |
| 24 | L | 543 | 0.7 | 0.692 | 28.5 | LOS C | 129 | 0.87 | 0.86 | 33.6 |
| 26 | R | 200 | 1.0 | 0.667 | 44.3 | LOS D | 67 | 0.99 | 0.85 | 27.0 |
| Approach |  | 743 | 0.8 | 0.692 | 32.8 | LOS C | 129 | 0.91 | 0.86 | 31.5 |
| Old Northern Rd (NW) |  |  |  |  |  |  |  |  |  |  |
| 27 | L | 430 | 0.9 | 0.701 | 31.8 | LOS C | 110 | 0.89 | 0.86 | 32.0 |
| 28 | T | 941 | 4.9 | 0.586 | 19.1 | LOS B | 112 | 0.82 | 0.72 | 39.4 |
| Approach |  | 1371 | 3.6 | 0.701 | 23.1 | LOS B | 112 | 0.84 | 0.76 | 36.7 |
| All Vehicles |  | 2905 | 4.6 | 0.701 | 23.6 | LOS B | 129 | 0.80 | 0.73 | 36.4 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> $(\mathbf{m})$ | Prop. <br> Queued | Eff. Stop <br> Rate |
| ---: | ---: | ---: | :---: | :---: | :---: | :---: |
| P9 | 53 | 33.3 | LOS D | 0 | 0.91 | 0.91 |
| P11 | 53 | 16.9 | LOS B | 0 | 0.65 | 0.65 |
| P13 | 53 | 33.3 | LOS D | 0 | 0.91 | 0.91 |
| All Peds | $\mathbf{1 5 9}$ | $\mathbf{2 7 . 8}$ | LOS B | $\mathbf{0}$ | $\mathbf{0 . 8 3}$ | $\mathbf{0 . 8 3}$ |

[^13][^14]
## Movement Summary

## Hastings Road / Old Northern Road

## 2018 PM peak

Signalised - Fixed time $\quad$ Cycle Time $=\mathbf{7 0}$ seconds

Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northern Rd (SE) |  |  |  |  |  |  |  |  |  |  |
| 22 | T | 1031 | 1.7 | 0.456 | 8.8 | LOS A | 82 | 0.61 | 0.54 | 48.3 |
| 23 | R | 319 | 0.3 | 0.822 | 42.2 | LOS C | 86 | 1.00 | 1.07 | 27.7 |
| Approach |  | 1350 | 1.4 | 0.822 | 16.7 | LOS B | 86 | 0.70 | 0.66 | 41.1 |
| Hastings Road |  |  |  |  |  |  |  |  |  |  |
| 24 | L | 464 | 0.2 | 0.531 | 22.3 | LOS B | 90 | 0.75 | 0.83 | 37.1 |
| 26 | R | 179 | 0.6 | 0.564 | 38.2 | LOS C | 53 | 0.97 | 0.81 | 29.2 |
| Approach |  | 643 | 0.3 | 0.564 | 26.7 | LOS B | 90 | 0.81 | 0.82 | 34.5 |
| Old Northern Rd (NW) |  |  |  |  |  |  |  |  |  |  |
| 27 | L | 449 | 0.2 | 0.848 | 41.3 | LOS C | 127 | 1.00 | 1.03 | 28.1 |
| 28 | T | 522 | 1.7 | 0.379 | 18.2 | LOS B | 60 | 0.79 | 0.66 | 40.0 |
| Approach |  | 971 | 1.0 | 0.848 | 28.9 | Los C | 127 | 0.89 | 0.83 | 33.4 |
| All Vehicles |  | 2964 | 1.0 | 0.848 | 22.9 | Los B | 127 | 0.79 | 0.75 | 36.8 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> $(\mathbf{m})$ | Prop. <br> Queued | Eff. Stop <br> Rate |
| ---: | ---: | ---: | :---: | :---: | :---: | :---: |
| P9 | 53 | 29.3 | LOS C | 0 | 0.91 | 0.91 |
| P11 | 53 | 18.6 | LOS B | 0 | 0.73 | 0.73 |
| P13 | 53 | 29.3 | LOS C | 0 | 0.91 | 0.91 |
| All Peds | $\mathbf{1 5 9}$ | $\mathbf{2 5 . 7}$ | LOS B | $\mathbf{0}$ | $\mathbf{0 . 8 5}$ | $\mathbf{0 . 8 5}$ |

[^15][^16]
## Movement Summary

## Old Northern Rd / New Line Rd

## 2018 AM peak

Roundabout
Vehicle Movements


Symbols which may appear in this table:
Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity


## Following LOS

\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

## Movement Summary

## Old Northern Rd / New Line Rd

## 2018 PM peak

## Roundabout

Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| New Line Road |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 57 | 8.8 | 0.500 | 9.7 | LOS A | 30 | 0.72 | 0.86 | 47.3 |
| 2 | T | 614 | 1.5 | 0.703 | 8.8 | LOS A | 62 | 0.76 | 0.84 | 47.8 |
| 3 | R | 492 | 1.8 | 0.703 | 16.7 | LOS B | 62 | 0.83 | 0.98 | 42.3 |
| Approach |  | 1163 | 2.0 | 0.703 | 12.2 | LOS A | 62 | 0.79 | 0.90 | 45.2 |
| Industrial Park |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 84 | 0.0 | 0.203 | 8.1 | LOS A | 10 | 0.71 | 0.71 | 47.4 |
| 5 | T | 130 | 3.1 | 0.203 | 7.2 | LOS A | 10 | 0.71 | 0.65 | 48.2 |
| 6 | R | 87 | 2.3 | 0.203 | 14.6 | LOS B | 9 | 0.71 | 0.90 | 43.7 |
| Approach |  | 301 | 2.0 | 0.203 | 9.6 | LOS A | 10 | 0.71 | 0.74 | 46.5 |
| Old Northern Rd N |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 47 | 4.3 | 0.522 | 9.6 | LOS A | 37 | 0.80 | 0.86 | 46.9 |
| 8 | T | 516 | 1.7 | 0.521 | 8.4 | LOS A | 37 | 0.80 | 0.80 | 47.5 |
| 9 | R | 359 | 3.6 | 0.520 | 16.0 | LOS B | 36 | 0.80 | 0.94 | 42.8 |
| Approach |  | 922 | 2.6 | 0.520 | 11.4 | LOS A | 37 | 0.80 | 0.86 | 45.5 |
| Old Northern Road w |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 405 | 0.5 | 0.419 | 9.1 | LOS A | 27 | 0.86 | 0.80 | 46.4 |
| 11 | T | 23 | 17.4 | 0.035 | 8.1 | LOS A | 2 | 0.74 | 0.70 | 48.0 |
| 12 | R | 125 | 1.6 | 0.171 | 14.5 | LOS B | 9 | 0.77 | 0.88 | 43.5 |
| Approach |  | 553 | 1.4 | 0.419 | 10.3 | LOS A | 27 | 0.84 | 0.82 | 45.7 |
| All Vehicles |  | 2939 | 2.1 | 0.703 | 11.3 | LOS A | 62 | 0.79 | 0.86 | 45.5 |

Symbols which may appear in this table:
Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity


## Following LOS

\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

## Movement Summary

## Kenthurst Rd / Old Northern Rd

## 2018 AM peak

Signalised - Fixed time $\quad$ Cycle Time $=\mathbf{8 0}$ seconds

Vehicle Movements


## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Quened | Eff. Stop <br> Rate |
| ---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P3 | 50 | 34.2 | LOS D | 0 | 0.93 | 0.93 |
| P5 | 50 | 24.8 | LOS C | 0 | 0.79 | 0.79 |
| All Peds | $\mathbf{1 0 0}$ | $\mathbf{2 9 . 5}$ | LOS C | $\mathbf{0}$ | $\mathbf{0 . 8 6}$ | $\mathbf{0 . 8 6}$ |

[^17]Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity


## Following LOS

\# - Based on density for continuous movements

## Movement Summary

## Kenthurst Rd / Old Northern Rd

## 2018 PM peak

Signalised - Fixed time $\quad$ Cycle Time $=\mathbf{9 0}$ seconds
Vehicle Movements


## Pedestrian Movements

|  |  |  |  |  |  |  |
| ---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | Back of <br> Queue <br> (m) | Prop. <br> Queued | Eff. Stop <br> Rate |
| P3 | 50 | 32.9 | LOS D | 0 | 0.86 | 0.86 |
| P5 | 50 | 35.6 | LOS D | 0 | 0.89 | 0.89 |
| All Peds | $\mathbf{1 0 0}$ | $\mathbf{3 4 . 2}$ | LOS C | $\mathbf{0}$ | $\mathbf{0 . 8 7}$ | $\mathbf{0 . 8 7}$ |

[^18]Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity


## Following LOS

\# - Based on density for continuous movements

## Movement Summary

## Hastings Road / New Line Road

## 2018 AM peak

Signalised - Fixed time $\quad$ Cycle Time $=\mathbf{8 0}$ seconds
Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| New Line Rd (south) |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 439 | 0.5 | 0.374 | 12.0 | LOS A | 52 | 0.37 | 0.75 | 45.1 |
| 2 | T | 939 | 5.8 | 0.740 | 26.9 | LOS B | 132 | 0.94 | 0.86 | 34.5 |
| Approach |  | 1378 | 4.1 | 0.740 | 22.2 | LOS B | 132 | 0.76 | 0.82 | 37.3 |
| New Line Rd (north) |  |  |  |  |  |  |  |  |  |  |
| 8 | T | 971 | 6.1 | 0.518 | 0.1 | LOS D\# | 17\# | 0.00 | 0.00 | 59.8 |
| 9 | R | 309 | 0.3 | 0.741 | 49.5 | LOS D | 57 | 1.00 | 0.89 | 25.4 |
| Approach |  | 1280 | 4.7 | 0.741 | 12.0 | LOS A | 57 | 0.24 | 0.21 | 45.1 |
| Hastings Rd |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 224 | 0.9 | 0.237 | 19.8 | LOS B | 45 | 0.59 | 0.77 | 38.8 |
| 12 | R | 449 | 1.3 | 0.751 | 36.3 | LOS C | 125 | 0.95 | 0.91 | 30.0 |
| Approach |  | 673 | 1.2 | 0.751 | 30.8 | LOS C | 125 | 0.83 | 0.86 | 32.5 |
| All Vehicles |  | 3331 | 3.7 | 0.751 | 20.0 | LOS B | 132 | 0.58 | 0.60 | 38.7 |

Symbols which may appear in this table:
Following Degree of Saturation
\# x $=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements

Following Queue
\# - Density for continuous movement

## SIDRA SOLUTIONS

Site: 2018 AM 2 lanes NLR
$\mathrm{K}: \backslash 60044833$ _South_Dural\4. Tech work area \4.3. Engineering\Traffic \SIDRA Rd_v2_with dev.aap
Processed Aug 26, 2008 12:41:57PM
A0194, Maunsell Australia Pty Ltd, Large Office
Produced by SIDRA Intersection 3.2.0.1455
Copyright 2000-2007 Akcelik and Associates Pty Ltd

## Movement Summary

## Hastings Road / New Line Road

## 2018 PM peak

```
Signalised - Fixed time Cycle Time = 90 seconds
```

Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| New Line Rd (south) |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 329 | 0.6 | 0.280 | 11.4 | LOS A | 38 | 0.30 | 0.73 | 45.7 |
| 2 | T | 1105 | 3.9 | 0.817 | 32.9 | LOS C | 177 | 0.97 | 0.95 | 31.5 |
| Approach |  | 1434 | 3.1 | 0.817 | 28.0 | LOS B | 177 | 0.82 | 0.90 | 33.9 |
| New Line Rd (north) |  |  |  |  |  |  |  |  |  |  |
| 8 | T | 887 | 2.3 | 0.461 | 0.1 | LOS C\# | 15\# | 0.00 | 0.00 | 59.9 |
| 9 | R | 310 | 0.3 | 0.836 | 58.7 | LOS E | 65 | 1.00 | 0.97 | 22.9 |
| Approach |  | 1197 | 1.8 | 0.837 | 15.3 | LOS B | 65 | 0.26 | 0.25 | 42.2 |
| Hastings Rd |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 271 | 1.5 | 0.288 | 21.7 | LOS B | 60 | 0.61 | 0.78 | 37.6 |
| 12 | R | 521 | 0.8 | 0.819 | 42.1 | LOS C | 166 | 0.98 | 0.97 | 27.8 |
| Approach |  | 792 | 1.0 | 0.819 | 35.1 | LOS C | 166 | 0.85 | 0.90 | 30.5 |
| All Vehicles |  | 3423 | 2.2 | 0.836 | 25.2 | LOS B | 177 | 0.63 | 0.67 | 35.4 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements

Following Queue
\# - Density for continuous movement

## SIDRA SOLUTIONS

Site: 2018 PM 2 lanes NLR
$\mathrm{K}: \backslash 60044833$ _South_Dural $\backslash 4$. Tech work area \4.3. Engineering\Traffic $\backslash$ SIDRA Rd_v2_with dev.aap
Processed Aug 26, 2008 12:42:41PM
A0194, Maunsell Australia Pty Ltd, Large Office
Produced by SIDRA Intersection 3.2.0.1455
Copyright 2000-2007 Akcelik and Associates Pty Ltd

D R A F T

## Appendix E 2018 (with development) Traffic Flows and Intersection Layouts

D R A F T
2018 AM Peak (with development) Turning Flows

| Old Northern Road / Glenhaven Road / Access 2 | Old Northern Road / Gilbert Road / Access 1 | Old Northern Road Hastings Road |
| :---: | :---: | :---: |
| Old Northern Rd (north) <br> Old Northen Rd (south) |  |  |
| Old Northern Road / New Line Road | Old Northern Road / Kenthurst Road | New Line Road / Hastings Road |
|  |  |  |

[^19]D R A F T

K:I60044833_South_Dural|6. Draft docs|6.1. Reports\South Dural TIA_150109_Rev C.doc
Revision C 15 January 2009
D R A F T
2018 PM Peak (with development) Turning Flows

| Old Northern Road / Glenhaven Road / Access 2 | Old Northern Road / Gilbert Road / Access 1 | Old Northern Road Hastings Road |
| :---: | :---: | :---: |
| Old Northern Rd (north) <br> Old Northen Rd (south) |  |  |
| Old Northern Road / New Line Road | Old Northern Road / Kenthurst Road / Access 4 | w Line Road / Hastings Road |
|  |  |  |

South Dural Urban Release Area
Revision C 15 January 2009
DRAFT

K:l60044833_South_Durall6. Draft docs 16.1 . Reports\South Dural TIA_150109_Rev C.doc

[^20]D R A F T
2018 intersection layout (with development)
Old Northern Road / Hastings Road

New Line Road / Hastings Road
 Old Northern Road / Kenthurst Road
South Dural Urban Release Area
K:I60044833_South_Durall/6. Draft docs\6.1. Reports\South Dural TIA_150109_Rev C.doc
Revision C 15 January 2009
D R A F T
New Line Road / Access to Bunnings /
Access 5





[^21]|

New Line Ro

K:160044833_South_Durall6. Draft docs16.1. Reports\South Dural TIA_150109_Rev C.doc

D R A F T

Appendix F 2018 (with development) SIDRA Results

## Movement Summary

## Glenhaven Road / Old Northern Road

## 2018 AM peak with development

```
Signalised - Fixed time Cycle Time = 110 seconds
```

Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver <br> Delay <br> (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northen Rd (south) |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 167 | 4.2 | 0.249 | 21.4 | LOS B | 33 | 0.73 | 0.78 | 37.8 |
| 2 | T | 718 | 10.3 | 0.772 | 43.9 | LOS D | 150 | 0.99 | 0.91 | 27.2 |
| Approach |  | 885 | 9.2 | 0.772 | 39.7 | LOS C | 150 | 0.94 | 0.88 | 28.7 |
| Access Road |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 11 | 0.0 | 0.054 | 56.3 | LOS D | 6 | 0.92 | 0.68 | 23.5 |
| 5 | T | 49 | 0.0 | 0.302 | 45.3 | LOS D | 41 | 0.93 | 0.73 | 26.7 |
| 6 | R | 40 | 0.0 | 0.302 | 53.4 | LOS D | 41 | 0.93 | 0.77 | 24.3 |
| Approach |  | 100 | 0.0 | 0.302 | 49.8 | LOS D | 41 | 0.93 | 0.74 | 25.3 |
| Old Northern Rd (north) |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 10 | 0.0 | 0.573 | 43.6 | LOS D | 272 | 0.74 | 0.85 | 27.2 |
| 8 | T | 1132 | 4.5 | 0.582 | 27.4 | LOS B | 272 | 0.74 | 0.67 | 34.2 |
| 9 | R | 274 | 11.3 | 0.763 | 56.2 | LOS D | 122 | 1.00 | 0.91 | 23.6 |
| Approach |  | 1416 | 5.8 | 0.763 | 33.1 | LOS C | 272 | 0.79 | 0.72 | 31.5 |
| Glenhaven Rd |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 461 | 5.2 | 0.773 | 17.1 | LOS B | 86 | 0.45 | 0.76 | 41.1 |
| 11 | T | 12 | 0.0 | 0.673 | 48.4 | LOS D | 87 | 0.99 | 0.84 | 25.7 |
| 12 | R | 189 | 3.2 | 0.673 | 56.6 | LOS E | 87 | 0.99 | 0.84 | 23.5 |
| Approach |  | 662 | 4.5 | 0.773 | 29.0 | LOS C | 87 | 0.62 | 0.79 | 33.6 |
| All Vehicles |  | 3063 | 6.3 | 0.773 | 34.6 | LOS C | 272 | 0.80 | 0.78 | 30.8 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P1 | 50 | 49.2 | LOS E | 0 | 0.95 | 0.95 |
| P3 | 50 | 35.2 | LOS D | 0 | 0.80 | 0.80 |
| P5 | 50 | 48.2 | LOS E | 0 | 0.94 | 0.94 |
| P7 | 50 | 35.2 | LOS D | 0 | 0.80 | 0.80 |

## Movement Summary

## Glenhaven Road / Old Northern Road

## 2018 PM peak with development

Signalised - Fixed time
Cycle Time $=130$ seconds
Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northen Rd (south) |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 428 | 0.9 | 0.670 | 29.1 | LOS C | 110 | 0.91 | 0.85 | 33.3 |
| 2 | T | 838 | 2.6 | 0.861 | 58.1 | LOS E | 202 | 1.00 | 1.00 | 23.1 |
| Approach |  | 1266 | 2.1 | 0.861 | 48.3 | LOS D | 202 | 0.97 | 0.95 | 25.8 |
| Access Road |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 3 | 0.0 | 0.018 | 66.4 | LOS E | 2 | 0.93 | 0.64 | 21.2 |
| 5 | T | 12 | 0.0 | 0.088 | 53.9 | LOS D | 13 | 0.91 | 0.66 | 24.2 |
| 6 | R | 10 | 0.0 | 0.088 | 62.1 | LOS E | 13 | 0.91 | 0.71 | 22.1 |
| Approach |  | 25 | 0.0 | 0.088 | 58.7 | LOS E | 13 | 0.91 | 0.68 | 22.9 |
| Old Northern Rd (north) |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 40 | 0.0 | 0.291 | 24.5 | LOS B | 81 | 0.52 | 0.79 | 35.8 |
| 8 | T | 628 | 3.0 | 0.291 | 14.7 | LOS B | 84 | 0.52 | 0.46 | 42.8 |
| 9 | R | 493 | 0.4 | 0.887 | 67.0 | LOS E | 240 | 1.00 | 1.04 | 21.1 |
| Approach |  | 1161 | 1.8 | 0.887 | 37.2 | LOS C | 240 | 0.72 | 0.72 | 29.6 |
| Glenhaven Rd |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 300 | 1.7 | 0.553 | 13.2 | LOS A | 52 | 0.39 | 0.70 | 44.2 |
| 11 | T | 49 | 0.0 | 0.500 | 57.9 | LOS E | 64 | 0.98 | 0.78 | 23.2 |
| 12 | R | 74 | 1.4 | 0.500 | 66.0 | LOS E | 64 | 0.98 | 0.79 | 21.3 |
| Approach |  | 423 | 1.4 | 0.552 | 27.6 | LOS B | 64 | 0.56 | 0.73 | 34.2 |
| All Vehicles |  | 2875 | 1.8 | 0.887 | 40.9 | LOS C | 240 | 0.81 | 0.82 | 28.3 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> $(\mathbf{m})$ | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P1 | 10 | 59.1 | LOS E | 0 | 0.95 | 0.95 |
| P3 | 10 | 40.8 | LOS E | 0 | 0.79 | 0.79 |
| P5 | 10 | 59.1 | LOS E | 0 | 0.95 | 0.95 |
| P7 | 10 | 40.8 | LOS E | 0 | 0.79 | 0.79 |

## Movement Summary

## Gilbert Road / Old Northern Road

## 2018 AM peak with development

Signalised - Fixed time $\quad$ Cycle Time $=110$ seconds

Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northern Road (SE) |  |  |  |  |  |  |  |  |  |  |
| 21 | L | 261 | 2.7 | 0.481 | 29.3 | LOS C | 76 | 0.69 | 0.79 | 33.2 |
| 22 | T | 509 | 13.2 | 0.866 | 58.5 | LOS E | 128 | 1.00 | 1.03 | 23.0 |
| 23 | R | 46 | 0.0 | 0.639 | 71.0 | LOS F | 27 | 1.00 | 0.76 | 20.3 |
| Approach |  | 816 | 9.1 | 0.866 | 49.9 | LOS D | 128 | 0.90 | 0.94 | 25.3 |
| Access Road |  |  |  |  |  |  |  |  |  |  |
| 24 | L | 172 | 0.0 | 0.800 | 62.9 | LOS E | 79 | 1.00 | 0.93 | 21.9 |
| 25 | T | 111 | 0.0 | 0.211 | 41.7 | LOS C | 33 | 0.89 | 0.69 | 28.0 |
| 26 | R | 37 | 0.0 | 0.211 | 49.8 | LOS D | 33 | 0.89 | 0.76 | 25.3 |
| Approach |  | 320 | 0.0 | 0.800 | 54.0 | LOS D | 79 | 0.95 | 0.83 | 24.1 |
| Old Northern Road (NW) |  |  |  |  |  |  |  |  |  |  |
| 27 | L | 1 | 0.0 | 0.809 | 34.4 | LOS C | 165 | 0.85 | 0.86 | 30.8 |
| 28 | T | 1063 | 3.9 | 0.655 | 26.5 | LOS B | 166 | 0.85 | 0.76 | 34.7 |
| 29 | R | 331 | 2.7 | 0.869 | 63.8 | LOS E | 147 | 1.00 | 1.03 | 21.8 |
| Approach |  | 1395 | 3.6 | 0.869 | 35.4 | LOS C | 166 | 0.89 | 0.82 | 30.4 |
| Gilbert Road |  |  |  |  |  |  |  |  |  |  |
| 30 | L | 239 | 5.4 | 0.547 | 27.7 | LOS B | 70 | 0.89 | 0.82 | 34.1 |
| 31 | T | 37 | 0.0 | 0.548 | 19.3 | LOS B | 70 | 0.89 | 0.75 | 39.2 |
| 32 | R | 700 | 1.3 | 0.837 | 58.6 | LOS E | 146 | 1.00 | 0.98 | 23.0 |
| Approach |  | 976 | 2.3 | 0.837 | 49.6 | LOS D | 146 | 0.97 | 0.93 | 25.4 |
| All Vehicles |  | 3507 | 4.2 | 0.869 | 44.4 | Los D | 166 | 0.92 | 0.88 | 27.0 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Queued | Eff. Stop <br> Rate |
| ---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P9 | 50 | 49.2 | LOS E | 0 | 0.95 | 0.95 |
| P11 | 50 | 48.2 | LOS E | 0 | 0.94 | 0.94 |
| P13 | 50 | 41.9 | LOS E | 0 | 0.87 | 0.87 |

## Movement Summary

## Gilbert Road / Old Northern Road

## 2018 PM peak with development

Signalised - Fixed time Cycle Time $=\mathbf{1 3 0}$ seconds
Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northern Road (SE) |  |  |  |  |  |  |  |  |  |  |
| 21 | L | 357 | 1.1 | 0.451 | 19.5 | LOS B | 64 | 0.67 | 0.80 | 39.0 |
| 22 | T | 965 | 2.2 | 0.815 | 48.2 | LOS D | 211 | 0.99 | 0.93 | 25.8 |
| 23 | R | 184 | 0.0 | 0.758 | 70.8 | LOS F | 94 | 1.00 | 0.89 | 20.4 |
| Approach |  | 1506 | 1.7 | 0.816 | 44.2 | LOS D | 211 | 0.92 | 0.89 | 27.1 |
| New NE leg |  |  |  |  |  |  |  |  |  |  |
| 24 | L | 43 | 0.0 | 0.128 | 29.5 | LOS C | 13 | 0.80 | 0.73 | 33.1 |
| 25 | T | 28 | 0.0 | 0.062 | 50.6 | LOS D | 11 | 0.88 | 0.63 | 25.1 |
| 26 | R | 9 | 0.0 | 0.062 | 58.7 | LOS E | 11 | 0.88 | 0.71 | 23.0 |
| Approach |  | 80 | 0.0 | 0.128 | 40.2 | LOS C | 13 | 0.84 | 0.69 | 28.5 |
| Old Northern Road (NW) |  |  |  |  |  |  |  |  |  |  |
| 27 | L | 1 | 0.0 | 0.426 | 46.3 | LOS D | 107 | 0.85 | 0.82 | 26.3 |
| 28 | T | 527 | 5.3 | 0.455 | 38.5 | LOS C | 108 | 0.85 | 0.73 | 29.2 |
| 29 | R | 201 | 1.0 | 0.834 | 75.0 | LOS F | 106 | 1.00 | 0.96 | 19.6 |
| Approach |  | 729 | 4.1 | 0.834 | 48.6 | LOS D | 108 | 0.89 | 0.79 | 25.7 |
| Gilbert Road |  |  |  |  |  |  |  |  |  |  |
| 30 | L | 263 | 0.8 | 0.829 | 61.9 | LOS E | 146 | 0.96 | 0.96 | 22.2 |
| 31 | T | 148 | 0.0 | 0.828 | 56.2 | LOS D | 162 | 0.98 | 0.95 | 23.6 |
| 32 | R | 596 | 0.8 | 0.828 | 65.8 | LOS E | 162 | 1.00 | 0.96 | 21.4 |
| Approach |  | 1007 | 0.7 | 0.829 | 63.3 | LOS E | 162 | 0.99 | 0.96 | 21.9 |
| All Vehicles |  | 3322 | 1.9 | 0.834 | 50.8 | LOS D | 211 | 0.93 | 0.88 | 25.0 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Queued | Eff. Stop <br> Rate |
| ---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P9 | 50 | 59.1 | LOS E | 0 | 0.95 | 0.95 |
| P11 | 50 | 39.2 | LOS D | 0 | 0.78 | 0.78 |
| P13 | 50 | 48.2 | LOS E | 0 | 0.86 | 0.86 |

## Movement Summary

## Hastings Road / Old Northern Road

## 2018 AM peak with development

Signalised - Fixed time Cycle Time $=\mathbf{1 0 0}$ seconds

Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Old Northern Rd (SE) |  |  |  |  |  |  |  |  |  |  |
| 22 | T | 605 | 11.7 | 0.242 | 6.1 | LOS A | 54 | 0.40 | 0.35 | 51.4 |
| 23 | R | 317 | 1.9 | 0.824 | 55.2 | LOS D | 124 | 1.00 | 0.97 | 23.8 |
| Approach |  | 922 | 8.4 | 0.824 | 23.0 | LOS B | 124 | 0.61 | 0.56 | 36.8 |
| Hastings Road |  |  |  |  |  |  |  |  |  |  |
| 24 | L | 695 | 0.6 | 0.916 | 56.9 | LOS E | 289 | 1.00 | 1.14 | 23.4 |
| 26 | R | 217 | 0.9 | 0.840 | 61.1 | LOS E | 93 | 1.00 | 0.98 | 22.4 |
| Approach |  | 912 | 0.7 | 0.917 | 57.9 | LOS E | 289 | 1.00 | 1.11 | 23.1 |
| Old Northern Rd (NW) |  |  |  |  |  |  |  |  |  |  |
| 27 | L | 494 | 0.8 | 0.926 | 44.4 | LOS D | 166 | 0.92 | 0.94 | 27.0 |
| 28 | T | 1385 | 3.3 | 0.864 | 36.2 | LOS C | 251 | 0.98 | 1.00 | 30.1 |
| Approach |  | 1879 | 2.7 | 0.926 | 38.4 | LOS C | 251 | 0.97 | 0.99 | 29.2 |
| All Vehicles |  | 3713 | 3.6 | 0.926 | 39.3 | LOS C | 289 | 0.88 | 0.91 | 28.8 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Queued | Eff. Stop <br> Rate |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| P9 | 53 | 42.3 | LOS E | 0 | 0.92 | 0.92 |
| P11 | 53 | 20.5 | LOS C | 0 | 0.64 | 0.64 |
| P13 | 53 | 42.3 | LOS E | 0 | 0.92 | 0.92 |
| All Peds | $\mathbf{1 5 9}$ | $\mathbf{3 5 . 0}$ | LOS C | $\mathbf{0}$ | $\mathbf{0 . 8 3}$ | $\mathbf{0 . 8 3}$ |

[^22][^23]
## Movement Summary

## Hastings Road / Old Northern Road

## 2018 PM peak with development

Signalised - Fixed time Cycle Time $=\mathbf{1 0 0}$ seconds
Vehicle Movements


## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Queued | Eff. Stop <br> Rate |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| P9 | 53 | 43.2 | LOS E | 0 | 0.93 | 0.93 |
| P11 | 53 | 28.1 | LOS C | 0 | 0.75 | 0.75 |
| P13 | 53 | 43.2 | LOS E | 0 | 0.93 | 0.93 |
| All Peds | $\mathbf{1 5 9}$ | $\mathbf{3 8 . 2}$ | LOS C | $\mathbf{0}$ | $\mathbf{0 . 8 7}$ | $\mathbf{0 . 8 7}$ |

[^24][^25]
## Movement Summary

## Old Northern Rd / New Line Rd

## 2018 AM peak with development

Signalised - Fixed time Cycle Time $=\mathbf{1 0 0}$ seconds

Vehicle Movements


Symbols which may appear in this table:
Following Degree of Saturation
$\# x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity


## Following LOS

\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

## Movement Summary

## Old Northern Rd / New Line Rd

## 2018 AM peak with development

## Roundabout

Vehicle Movements


Symbols which may appear in this table:
Following Degree of Saturation
$\# x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity


## Following LOS

\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

## Movement Summary

## Old Northern Rd / New Line Rd

## 2018 PM peak with development

Signalised - Fixed time $\quad$ Cycle Time $=70$ seconds

Vehicle Movements


Symbols which may appear in this table:
Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity


## Following LOS

\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

## Movement Summary

## Old Northern Rd / New Line Rd

## 2018 PM peak with development

## Roundabout

Vehicle Movements


Symbols which may appear in this table:
Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity


## Following LOS

\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

## Movement Summary

## Kenthurst Rd / Old Northern Rd

## 2018 AM peak with development

Signalised - Fixed time Cycle Time $=\mathbf{9 0}$ seconds

Vehicle Movements


## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> $(\mathbf{m})$ | Prop. <br> Queued | Eff. Stop <br> Rate |
| ---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P3 | 50 | 38.3 | LOS D | 0 | 0.92 | 0.92 |
| P5 | 50 | 24.2 | LOS C | 0 | 0.73 | 0.73 |
| All Peds | $\mathbf{1 0 0}$ | $\mathbf{3 1 . 2}$ | LOS C | $\mathbf{0}$ | $\mathbf{0 . 8 3}$ | $\mathbf{0 . 8 3}$ |

[^26]Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity


## Following LOS

\# - Based on density for continuous movements

## Movement Summary

## Kenthurst Rd / Old Northern Rd

## 2018 PM peak with development

```
Signalised - Fixed time Cycle Time = 150 seconds
```

Vehicle Movements


## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Quened | Eff. Stop <br> Rate |
| ---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P3 | 50 | 38.9 | LOS D | 0 | 0.72 | 0.72 |
| P5 | 50 | 59.9 | LOS E | 0 | 0.89 | 0.89 |
| All Peds | $\mathbf{1 0 0}$ | $\mathbf{4 9 . 4}$ | LOS D | $\mathbf{0}$ | $\mathbf{0 . 8 1}$ | $\mathbf{0 . 8 1}$ |

[^27]Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity


## Following LOS

\# - Based on density for continuous movements

## Movement Summary

## Hastings Road / New Line Road

## 2018 AM peak with development

```
Signalised - Fixed time Cycle Time = 80 seconds
```

Vehicle Movements


Symbols which may appear in this table:
Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements

Following Queue
\# - Density for continuous movement

## SIDRA SOLUTIONS

Site: 2018 AM 2 lanes NLR + dev
$\mathrm{K}: \backslash 60044833$ South_Dural\4. Tech work area\4.3. Engineering\Traffic \SIDRA\with dev traffic\revised analysis 120109 \Hastings Rd_New Line Rd_v2_with dev.aap
Processed Jan 13, 2009 11:27:30AM
A0194, Maunsell Australia Pty Ltd, Large Office
Produced by SIDRA Intersection 3.2.0.1455
Copyright 2000-2007 Akcelik and Associates Pty Ltd

## Movement Summary

## Hastings Road / New Line Road

## 2018 PM peak with development

Signalised - Fixed time Cycle Time $=120$ seconds

Vehicle Movements


Symbols which may appear in this table:
Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements

Following Queue
\# - Density for continuous movement

## SIDRA SOLUTIONS

Site: 2018 PM 2 lanes NLR + dev
$\mathrm{K}: \backslash 60044833$ South_Dural\4. Tech work area\4.3. Engineering\Traffic \SIDRA\with dev traffic\revised analysis 120109 \Hastings Rd_New Line Rd_v2_with dev.aap
Processed Jan 13, 2009 11:28:28AM
A0194, Maunsell Australia Pty Ltd, Large Office
Produced by SIDRA Intersection 3.2.0.1455
Copyright 2000-2007 Akcelik and Associates Pty Ltd

## Movement Summary

## Access 3

## 2018 AM peak with development

Signalised - Fixed time $\quad$ Cycle Time $=\mathbf{9 0}$ seconds
Vehicle Movements


## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> $(\mathbf{m})$ | Prop. <br> Queued | Eff. Stop <br> Rate |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| P1 | 53 | 37.4 | LOS D | 0 | 0.91 | 0.91 |
| P3 | 53 | 6.4 | LOS A | 0 | 0.38 | 0.38 |
| P5 | 53 | 34.7 | LOS D | 0 | 0.88 | 0.88 |
| All Peds | $\mathbf{1 5 9}$ | $\mathbf{2 6 . 1}$ | LOS B | $\mathbf{0}$ | $\mathbf{0 . 7 2}$ | $\mathbf{0 . 7 2}$ |

[^28][^29]
## Movement Summary

## Access 3

## 2018 PM peak with development

Signalised - Fixed time

$$
\text { Cycle Time }=100 \text { seconds }
$$

Vehicle Movements


## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> $(\mathbf{m})$ | Prop. <br> Queued | Eff. Stop <br> Rate |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| P1 | 53 | 44.2 | LOS E | 0 | 0.94 | 0.94 |
| P3 | 53 | 13.0 | LOS B | 0 | 0.51 | 0.51 |
| P5 | 53 | 41.4 | LOS E | 0 | 0.91 | 0.91 |
| All Peds | $\mathbf{1 5 9}$ | $\mathbf{3 2 . 9}$ | LOS C | $\mathbf{0}$ | $\mathbf{0 . 7 9}$ | $\mathbf{0 . 7 9}$ |

[^30][^31]
## Movement Summary

## Old Northern Road / Access 4

## 2018 AM peak with development

Signalised - Fixed time $\quad$ Cycle Time $=\mathbf{8 0}$ seconds

Vehicle Movements


## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Queued | Eff. Stop <br> Rate |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| P1 | 53 | 21.8 | LOS C | 0 | 0.74 | 0.74 |
| P3 | 53 | 13.8 | LOS B | 0 | 0.59 | 0.59 |
| P5 | 53 | 19.6 | LOS B | 0 | 0.70 | 0.70 |
| All Peds | $\mathbf{1 5 9}$ | $\mathbf{1 8 . 4}$ | LOS B | $\mathbf{0}$ | $\mathbf{0 . 6 7}$ | $\mathbf{0 . 6 7}$ |

[^32][^33]
## Movement Summary

## Old Northern Road / Access 4

## 2018 PM peak with development

Signalised - Fixed time Cycle Time $=120$ seconds
Vehicle Movements


## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> $(\mathbf{m})$ | Prop. <br> Queued | Eff. Stop <br> Rate |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| P1 | 53 | 54.1 | LOS E | 0 | 0.95 | 0.95 |
| P3 | 53 | 8.1 | LOS A | 0 | 0.37 | 0.37 |
| P5 | 53 | 51.3 | LOS E | 0 | 0.93 | 0.93 |
| All Peds | $\mathbf{1 5 9}$ | $\mathbf{3 7 . 9}$ | LOS C | $\mathbf{0}$ | $\mathbf{0 . 7 5}$ | $\mathbf{0 . 7 5}$ |

[^34][^35]
## Movement Summary

## New Line Road / Access 5

## 2018 AM peak with development

## Roundabout

Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| New Line Road S |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 34 | 0.0 | 0.540 | 6.6 | LOS A | 48 | 0.15 | 0.51 | 50.0 |
| 2 | T | 1321 | 29.0 | 0.539 | 6.3 | LOS A | 48 | 0.16 | 0.44 | 51.2 |
| 3 | R | 28 | 28.6 | 0.538 | 12.1 | LOS A | 48 | 0.16 | 0.64 | 45.9 |
| Approach |  | 1383 | 28.3 | 0.539 | 6.4 | LOS A | 48 | 0.16 | 0.44 | 51.0 |
| Industrial Estate |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 19 | 21.1 | 0.029 | 10.3 | LOS A | 1 | 0.62 | 0.74 | 47.4 |
| 5 | T | 1 | 0.0 | 0.029 | 8.6 | LOS A | 1 | 0.62 | 0.68 | 48.1 |
| 6 | R | 11 | 9.1 | 0.018 | 15.1 | LOS B | 1 | 0.62 | 0.78 | 43.4 |
| Approach |  | 31 | 16.1 | 0.029 | 11.9 | LOS A | 1 | 0.62 | 0.75 | 45.9 |
| New Line Road $\mathbf{N}$ |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 17 | 17.6 | 0.395 | 7.8 | LOS A | 22 | 0.40 | 0.59 | 48.5 |
| 8 | T | 971 | 0.0 | 0.392 | 6.2 | LOS A | 22 | 0.41 | 0.53 | 49.4 |
| 9 | R | 4 | 0.0 | 0.400 | 12.0 | LOS A | 22 | 0.41 | 0.68 | 44.9 |
| Approach |  | 992 | 0.3 | 0.392 | 6.2 | LOS A | 22 | 0.41 | 0.53 | 49.4 |
| Access Road (5) |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 17 | 0.0 | 0.037 | 12.5 | LOS A | 1 | 0.67 | 0.83 | 44.6 |
| 11 | T | 1 | 0.0 | 0.037 | 11.4 | LOS A | 1 | 0.67 | 0.80 | 45.7 |
| 12 | R | 137 | 0.0 | 0.178 | 15.4 | LOS B | 7 | 0.70 | 0.91 | 42.9 |
| Approach |  | 155 | 0.0 | 0.178 | 15.0 | LOS B | 7 | 0.69 | 0.90 | 43.1 |
| All Vehicles |  | 2561 | 15.6 | 0.540 | 6.9 | LOS A | 48 | 0.29 | 0.51 | 49.7 |

Symbols which may appear in this table:
Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity


## Following LOS

\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

## Movement Summary

## New Line Road / Access 5

## 2018 PM peak with development

## Roundabout

Vehicle Movements


Symbols which may appear in this table:
Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity


## Following LOS

\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

## Movement Summary

## New Line Road / Access 6

## 2018 AM peak with development

Signalised - Fixed time $\quad$ Cycle Time $=\mathbf{6 0}$ seconds

Vehicle Movements

| Mov ID Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| New Line Road S |  |  |  |  |  |  |  |  |  |
| 1 L | 59 | 0.0 | 0.734 | 22.1 | LOS B | 127 | 0.85 | 0.89 | 37.3 |
| 2 T | 1316 | 4.3 | 0.735 | 13.9 | LOS A | 130 | 0.85 | 0.79 | 43.4 |
| 3 R | 29 | 0.0 | 0.147 | 26.1 | LOS B | 7 | 0.76 | 0.74 | 34.9 |
| Approach | 1404 | 4.1 | 0.735 | 14.5 | LOS A | 130 | 0.85 | 0.79 | 42.9 |
| Sebastian Drive |  |  |  |  |  |  |  |  |  |
| 4 L | 116 | 0.0 | 0.290 | 29.3 | LOS C | 29 | 0.86 | 0.78 | 33.2 |
| 5 T | 1 | 0.0 | 0.303 | 21.1 | LOS B | 29 | 0.86 | 0.69 | 37.9 |
| 6 R | 68 | 0.0 | 0.160 | 29.3 | LOS C | 17 | 0.85 | 0.75 | 33.2 |
| Approach | 185 | 0.0 | 0.290 | 29.3 | LOS C | 29 | 0.86 | 0.77 | 33.2 |
| New Line Road $\mathbf{N}$ |  |  |  |  |  |  |  |  |  |
| 7 L | 17 | 0.0 | 0.583 | 19.7 | LOS B | 93 | 0.76 | 0.83 | 38.9 |
| 8 T | 1103 | 1.7 | 0.584 | 11.5 | LOS A | 94 | 0.76 | 0.67 | 45.6 |
| 9 R | 7 | 0.0 | 0.044 | 29.7 | LOS C | 2 | 0.82 | 0.69 | 33.0 |
| Approach | 1127 | 1.7 | 0.584 | 11.8 | LOS A | 94 | 0.76 | 0.67 | 45.3 |
| Access Road (6) |  |  |  |  |  |  |  |  |  |
| 10 L | 28 | 0.0 | 0.072 | 28.0 | LOS B | 7 | 0.81 | 0.72 | 33.9 |
| 11 T | 1 | 0.0 | 0.071 | 19.8 | LOS B | 7 | 0.81 | 0.59 | 38.9 |
| 12 R | 235 | 0.0 | 0.692 | 35.3 | LOS C | 61 | 0.99 | 0.88 | 30.4 |
| Approach | 264 | 0.0 | 0.692 | 34.5 | LOS C | 61 | 0.97 | 0.86 | 30.8 |
| All Vehicles | 2980 | 2.6 | 0.735 | 16.2 | LOS B | 130 | 0.82 | 0.75 | 41.5 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P1 | 50 | 23.4 | LOS C | 0 | 0.88 | 0.88 |
| P3 | 50 | 10.8 | LOS B | 0 | 0.60 | 0.60 |
| P5 | 50 | 23.4 | LOS C | 0 | 0.88 | 0.88 |

## Movement Summary

## New Line Road / Access 6

## 2018 PM peak with development

Signalised - Fixed time $\quad$ Cycle Time $=\mathbf{9 0}$ seconds

Vehicle Movements

| Mov ID Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| New Line Road S |  |  |  |  |  |  |  |  |  |
| 1 L | 235 | 0.0 | 0.776 | 19.2 | LOS B | 207 | 0.75 | 0.86 | 39.2 |
| 2 T | 1741 | 2.6 | 0.775 | 10.7 | LOS A | 212 | 0.74 | 0.69 | 46.3 |
| 3 R | 116 | 0.0 | 0.395 | 19.5 | LOS B | 28 | 0.56 | 0.77 | 39.0 |
| Approach | 2092 | 2.2 | 0.775 | 12.2 | LOS A | 212 | 0.73 | 0.71 | 45.0 |
| Sebastian Drive |  |  |  |  |  |  |  |  |  |
| 4 L | 29 | 0.0 | 0.120 | 45.3 | LOS D | 12 | 0.90 | 0.73 | 26.7 |
| 5 T | 1 | 0.0 | 0.120 | 37.1 | LOS C | 12 | 0.90 | 0.66 | 29.7 |
| 6 R | 17 | 0.0 | 0.064 | 45.5 | LOS D | 7 | 0.90 | 0.70 | 26.6 |
| Approach | 47 | 0.0 | 0.120 | 45.2 | LOS D | 12 | 0.90 | 0.71 | 26.7 |
| New Line Road $\mathbf{N}$ |  |  |  |  |  |  |  |  |  |
| 7 L | 68 | 0.0 | 0.320 | 14.9 | LOS B | 65 | 0.45 | 0.76 | 42.5 |
| 8 T | 741 | 5.3 | 0.321 | 6.5 | LOS A | 65 | 0.45 | 0.39 | 50.9 |
| 9 R | 28 | 0.0 | 0.287 | 36.4 | LOS C | 11 | 0.79 | 0.77 | 30.0 |
| Approach | 837 | 4.7 | 0.321 | 8.2 | LOS A | 66 | 0.46 | 0.44 | 49.0 |
| Access Road (6) |  |  |  |  |  |  |  |  |  |
| 10 L | 7 | 0.0 | 0.031 | 44.3 | LOS D | 3 | 0.88 | 0.67 | 27.0 |
| 11 T | 1 | 0.0 | 0.031 | 36.1 | LOS C | 3 | 0.88 | 0.60 | 30.1 |
| 12 R | 59 | 0.0 | 0.230 | 46.9 | LOS D | 24 | 0.93 | 0.75 | 26.2 |
| Approach | 67 | 0.0 | 0.230 | 46.5 | LOS D | 24 | 0.93 | 0.74 | 26.3 |
| All Vehicles | 3043 | 2.8 | 0.776 | 12.3 | LOS A | 212 | 0.67 | 0.64 | 44.8 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P1 | 50 | 39.2 | LOS D | 0 | 0.93 | 0.93 |
| P3 | 50 | 6.8 | LOS A | 0 | 0.39 | 0.39 |
| P5 | 50 | 39.2 | LOS D | 0 | 0.93 | 0.93 |


[^0]:    Source: Hornsby Shire Cycling Map 2008

[^1]:    Source: Inspire Urban Design and Planning, 2009

[^2]:    Source: Maunsell, 2008

[^3]:    South Dural Urban Release Area
    K:l60044833_South_Durall6. Draft docs16.1. Reports|South Dural TIA_150109_Rev C.doc
    Revision C 15 January 2009

[^4]:    South Dural Urban Release Area
    K:I60044833_South_Durall/6. Draft docsI6.1. Reports\South Dural TIA_150109_Rev C.doc
    K:I60044833_South_Durall6.
    Revision C 15 January 2009

[^5]:    Symbols which may appear in this table:

[^6]:    Symbols which may appear in this table:

[^7]:    South Dural Urban Release Area
    K:160044833_South_Durall6. Draft docs16.1. Reports|South Dural TIA_150109_Rev C.doc
    Revision C 15 January 2009

[^8]:    South Dural Urban Release Area
    K:160044833_South_Durall6. Draft docsI6.1. Reports\South Dural TIA_150109_Rev C.doc
    Revision C 15 January 2009

[^9]:    Symbols which may appear in this table:
    Following Degree of Saturation
    \# x = 1.00 for Short Lane with resulting Excess Flow

    * $x=1.00$ due to minimum capacity

[^10]:    Following LOS

[^11]:    Symbols which may appear in this table:
    Following Degree of Saturation
    $\# x=1.00$ for Short Lane with resulting Excess Flow

    * $x=1.00$ due to minimum capacity

[^12]:    Following LOS

[^13]:    Symbols which may appear in this table:
    Following Degree of Saturation
    $\# x=1.00$ for Short Lane with resulting Excess Flow

    * $x=1.00$ due to minimum capacity

[^14]:    Following LOS

[^15]:    Symbols which may appear in this table:
    Following Degree of Saturation
    $\# x=1.00$ for Short Lane with resulting Excess Flow

    * $x=1.00$ due to minimum capacity

[^16]:    Following LOS

[^17]:    Symbols which may appear in this table:

[^18]:    Symbols which may appear in this table:

[^19]:    South Dural Urban Release Area
    K: 160044833 South Durall6. Draft
    K:l60044833_South_Durall6. Draft docs\6.1. Reports\South Dural TIA_150109_Rev C.doc
    Revision C 15 January 2009 E-1

[^20]:    Revision C 15 January 2009

[^21]:    Old Northern Road / Access 3
    Old Northern Road / Access 4

[^22]:    Symbols which may appear in this table:
    Following Degree of Saturation
    \# x = 1.00 for Short Lane with resulting Excess Flow

    * $x=1.00$ due to minimum capacity

[^23]:    Following LOS

[^24]:    Symbols which may appear in this table:
    Following Degree of Saturation
    \# x = 1.00 for Short Lane with resulting Excess Flow

    * $x=1.00$ due to minimum capacity

[^25]:    Following LOS

[^26]:    Symbols which may appear in this table:

[^27]:    Symbols which may appear in this table:

[^28]:    Symbols which may appear in this table:
    Following Degree of Saturation
    $\# x=1.00$ for Short Lane with resulting Excess Flow

    * $x=1.00$ due to minimum capacity

[^29]:    Following LOS

[^30]:    Symbols which may appear in this table:
    Following Degree of Saturation
    \# x = 1.00 for Short Lane with resulting Excess Flow

    * $x=1.00$ due to minimum capacity

[^31]:    Following LOS

[^32]:    Symbols which may appear in this table:
    Following Degree of Saturation
    \# x = 1.00 for Short Lane with resulting Excess Flow

    * $x=1.00$ due to minimum capacity

[^33]:    Following LOS

[^34]:    Symbols which may appear in this table:
    Following Degree of Saturation
    \# x = 1.00 for Short Lane with resulting Excess Flow

    * $x=1.00$ due to minimum capacity

[^35]:    Following LOS

